

**January 17, 2023**  
**\*\*Workshop at 10:30\*\***  
**Resilient PSJ Grant**  
**Regular Meeting**  
**12:00 Noon**



## City of Port St. Joe

Rex Buzzett, Mayor-Commissioner  
Eric Langston, Commissioner, Group I  
David Ashbrook, Commissioner, Group II  
Brett Lowry, Commissioner, Group III  
Scott Hoffman, Commissioner, Group IV

[All persons are invited to attend these meetings. Any person who decides to appeal any decision made by the Commission with respect to any matter considered at said meeting will need a record of the proceedings, and for such purpose may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based. The Board of City Commission of the City of Port St. Joe, Florida will not provide a verbatim record of this meeting.]

# BOARD OF CITY COMMISSION

Regular Public Meeting  
12:00 Noon  
Tuesday January 17, 2023

## Call to Order

## Consent Agenda

### Minutes

- Regular Commission Meeting 12/6/22 Pages 4-7

### City Engineer

- Long Ave.
    - Water/Sewer
    - Paving
  - Ave. C & D Paving
    - Change Order #1
  - Dr. Joe Parking lot Paving
  - First Street Paving
  - Beacon Hill Sewer Design
- Page 8

### Planning Board Recommendations

- Development Order Request Pages 9-21
  - Rish Family Plaza  
Parcels 04830-006R thru 04830-016R
- Development Order Request Pages 22-96
  - St. Joe Company- Windmark Beach North Townhomes  
Parcel 04227-000R

### City Attorney

- Norton Family Properties, LLC- 4" Encroachment Pages 97-98

## Old Business

- City Projects Page 99

## New Business

- Trish Warriner- Garden Club Downtown Beautification Project
- David Warriner- Port Theatre Rebuild Status
- City License Requirements Pages 100-101

### Public Works

- Update

**Surface Water Plant**

- **Update**

**Wastewater Plant**

- **Update**

**Finance Director**

- **FEMA- Update**

**Code Enforcement**

- **Special Master Orders**
  - **Timothy Elder, 1310 Long Ave., Parcel 05171-000R** **Pages 102-105**
  - **James Padgett, 903 16<sup>th</sup> Street, Parcel # 05441-000R** **Pages 106-109**
  - **James Massey, 516 9<sup>th</sup> Street, Parcel #05103-000R** **Pages 110-122**

**Police Department**

- **Sick Leave Accrual for 12 Hour Shifts** **Page 123**

**City Clerk**

- **Grants Update** **Pages 124-125**

**Citizens to be Heard**

**Discussion Items by Commissioners**

**Motion to Adjourn**

# RESILIENT PORT ST. JOE COMMUNITY WORKSHOP #1

*January 17, 10:30-12:00*



The City of Port St. Joe is partnering with the University of Florida on a 2-year project to assess comprehensive flooding vulnerability, revise the comprehensive plan regarding flood risk, and conduct resilience planning for the City. This project is funded by FDEP's Resilient Florida Program and will set Port St. Joe up to access state funds in the coming years to address its flooding vulnerability. Attendees at this public meeting will help the project team by reflecting on community assets and vulnerabilities, reviewing draft map products, and providing input about their experiences with flooding in the city.

## **WHEN:**

*January 17, 10:30 - 12:00*

## **WHERE:**

**2775 Garrison Avenue, Port St. Joe**

## **Interested in Participating?**

Please contact Christian Calle for more information: [callefigueroac@ufl.edu](mailto:callefigueroac@ufl.edu)

RSVP not required to attend

**Organized and Supported by:**





**MINUTES OF THE REGULAR MEETING OF THE BOARD OF CITY  
COMMISSIONERS FOR THE CITY OF PORT ST. JOE, FLORIDA, HELD AT  
2775 GARRISON AVENUE, January 3, 2023, at Noon.**

The following were present: Mayor Buzzett, Commissioners Ashbrook, Hoffman, Langston, and Lowry. City Manager Jim Anderson, City Clerk Charlotte Pierce, and City Attorney Clinton McCahill were also present.

**CONSENT AGENDA**

**Minutes**

A Motion was made by Commissioner Ashbrook, second by Commissioner Langston, to approve the Minutes of the Regular Meeting of December 6, 2022. All in favor; Motion carried 5-0.

**City Engineer – Josh Baxley**

*Long Avenue Water / Sewer*

Dewberry is working with city staff on closeout documentation.

*Long Avenue Paving*

A Preconstruction Meeting was held before Christmas and the project is mobilizing today.

*Avenues C and D Paving*

Prior to Christmas, a Preconstruction Meeting was held, and the project is mobilizing today.

*Dr. Joe Parking Lot Paving –*

A Motion was made by Commissioner Ashbrook, second by Commissioner Lowry, to approve a Change Order in the amount of \$145,257 for North FL Construction to do the project. All in favor; Motion carried 5-0.

*First Street Paving –*

Mr. Baxley anticipates he will have a date of when the Inlets will arrive for the project.

*RFP 2022-19 Beacon Hill Lift Station:*

A Motion was made by Commissioner Ashbrook, second by Commissioner Hoffman, to accept the bid of Mainline Construction in the amount of \$824,778.65. All in favor; Motion carried 5-0.

*Buck Griffin Lake*

Mr. Anderson shared that a sandbar has appeared, and water levels are low in Buck Griffin Lake. After looking into the matter, it was discovered that the weir at the other end of the lake has been compromised. Mr. Baxley has spoken with several individuals about the issue and recommends that the area be cleaned up, place 57 stone in the water, use riprap, create a hole in the top of the weir and fill it with fillable flow. Rock will be added starting tomorrow.

## **City Attorney –**

### *Lobbying Firm Proposal*

Attorney McCahill shared that after researching this issue, there is no exemption for Lobbying in our Purchasing Policy. After speaking with state entities, waiving of the Purchasing Policy for this item is acceptable. A Motion was made by Commissioner Ashbrook, second by Commissioner Hoffman, for discussion. After a lengthy discussion, Commissioner Hoffman withdrew his second to the Motion. Commissioner Langston second the Motion to approve waiving the Purchasing Policy for this item. The Motion carried 3-2 with Commissioners Hoffman and Lowry voting no.

## **Old Business**

### *City Projects*

Mr. Anderson noted that there will be a lot of road construction being done on Long Avenue as well as Avenues C and D and asked for everyone's patience with the progress.

## **New Business –**

### *City License Requirements*

After discussion, this item was Tabled until the next meeting.

### *Pedestrian Crossing at Avenue A and Highway 98*

An email was received from Andy White, Safety Specialist with FDOT. They have reviewed the location and determined the safest crossing is at Marina Drive where the Traffic Signal and full pedestrian crossing features are located. There is currently sidewalk connectivity from Avenue A down to Marina Drive that doesn't require a pedestrian to cross US 98 and provides safe walking to the signalized intersection. FDOT can make a request to review this crossing again during the next resurfacing project.

Commissioner Langston asked that a call be made to FDOT requesting they reconsider their decision as he feels the crossing is needed at Avenue A rather than citizens having to walk to Marina Drive to cross.

## **Public Works – John Grantland**

### *Shop Building – Request to Bid*

A Motion was made by Commissioner Lowry, second by Commissioner Langston, to advertise for the construction of a shop building for Public Works. All in favor; Motion carried 5-0.

### *Surplus Property*

A Motion was made by Commissioner Lowry, second by Commissioner Ashbrook, to approve the surplus and sale of various pieces of scrap metal that Public Works has accumulated over the past six years from previous projects. All in favor; Motion carried 5-0. The scrap metal consists of replaced fire hydrants, cast iron piping, lift station rehabilitation piping and components, old manhole rings and covers, as well as non-functioning grinder pumps, etc.

### *Reid Avenue Tree Pruning*

Mr. Grantland has received a quote of \$2,000 to shape the trees on Reid Avenue. Before any work is done, a group consisting of city staff and the Garden Club will discuss the work to be done. The new Vac Truck was delivered prior to Christmas and is at Public Works should anyone wish to see it.

### **Surface Water Plant – Larry McClamma**

Mr. McClamma stated that the plant did not have any major issues from the freeze.

### **Wastewater Plant –**

Mr. Anderson shared that while Kevin Pettis is out, Joe Harris will be running the plant. Spraying is continuing and the city has received a modified permit from DEP for the Spray Fields.

### **Finance Director – Mike Lacour**

#### *FEMA Update*

Mr. Lacour will be following up with FEMA prior to the next meeting.

### **Code Enforcement –**

Mr. Anderson shared that at the next Code Enforcement Hearing, citizens that have not followed the Special Master's requirements will be addressed.

### **Police Department – Chief Richards**

A Motion was made by Commissioner Hoffman, second by Commissioner Ashbrook, for Chief Richards to apply for a \$6,045 grant to provide 10 body cams for officers. All in favor; Motion carried 5-0. There is a 50% match for the grant, but it can be covered by In-Kind services including training, IT Work, and writing policies for the use of the cameras. Maintaining of video files will be done by cloud service and will be \$43 per unit per month for 5 years.

### **City Clerk – Charlotte Pierce**

#### *Grants Update*

Clerk Pierce shared that there will be a conference call with our architect and grant managers concerning the Centennial Building Monday, January 9, at 2 P.M.

She also noted the Christmas Parade was a great success and thanked the Commissioners for their involvement with it.

### **Citizens to be Heard –**

Ed Long shared that the Christmas Parade was fantastic and invited the Commissioners to participate in the Polar Plunge next year.

### **Discussion Items by Commissioners**

*Neither Commissioners Langston nor Ashbrook* had anything else to share with the Commission.

*Commissioner Lowry* wished everyone a Happy New Year.

*Commissioner Hoffman* requested that the Buck Griffin Lake issue be added to the City Projects List and that the pipes under 20<sup>th</sup> Street be checked by cameras to see if the sand could be coming through them.

*Mayor Buzzett* noted the conference call concerning the Centennial Building Monday, January 9<sup>th</sup> at 2 PM, wished everyone a Happy New Year, and announced that he would be seeking another term as Mayor this year.

**Motion to Adjourn –**

There was no other business to come before the Commission and Mayor Buzzett adjourned the meeting at 1:10 P.M.

Approved this \_\_\_\_\_ day of \_\_\_\_\_ 2023.

\_\_\_\_\_  
Rex Buzzett, Mayor

\_\_\_\_\_  
Date

\_\_\_\_\_  
Charlotte M. Pierce, City Clerk

\_\_\_\_\_  
Date

Change Order No. 1

Date of Issuance:	Effective Date:
Owner: <b>City of Port St. Joe</b>	Owner's Contract No.:
Contractor: <b>American Sand and Asphalt, LLC</b>	Contractor's Project No.:
Engineer: <b>Dewberry Engineers Inc.</b>	Engineer's Project No.: <b>50146260</b>
Project: <b>Avenue C &amp; D Resurfacing Project</b>	Contract Name:

The Contract is modified as follows upon execution of this Change Order:

Description: Increase in Contract Price of \$17,390.00 due to Asphalt patch quantity increase.

Attachments: *[List documents supporting change]*

CHANGE IN CONTRACT PRICE	CHANGE IN CONTRACT TIMES <i>[note changes in Milestones if applicable]</i>
Original Contract Price:  \$ 449,499.70	Original Contract Times: Substantial Completion: <u>April 3, 2023 (90 days)</u> Ready for Final Payment: <u>May 3, 2023 (120 days)</u> days or dates
[Increase] [Decrease] from previously approved Change Orders No. ___ to No. ___:  \$ N/A	[Increase] [Decrease] from previously approved Change Orders No. ___ to No. ___: Substantial Completion: <u>N/A</u> Ready for Final Payment: _____ days
Contract Price prior to this Change Order:  \$ 449,499.70	Contract Times prior to this Change Order: Substantial Completion: <u>April 3, 2023 (90 days)</u> Ready for Final Payment: <u>May 3, 2023 (120 days)</u> days or dates
[Increase] [Decrease] of this Change Order:  \$ 17,390.00	[Increase] [Decrease] of this Change Order: Substantial Completion: <u>N/A</u> Ready for Final Payment: _____ days or dates
Contract Price incorporating this Change Order:  \$ 466,889.70	Contract Times with all approved Change Orders: Substantial Completion: <u>April 3, 2023 (90 days)</u> Ready for Final Payment: <u>May 3, 2023 (120 days)</u> days or dates

<b>RECOMMENDED:</b>		<b>ACCEPTED:</b>		<b>ACCEPTED:</b>	
By: _____	By: _____	By: _____	By: _____	By: _____	By: _____
Engineer (if required)	Owner (Authorized Signature)	Owner (Authorized Signature)	Contractor (Authorized Signature)	Contractor (Authorized Signature)	Contractor (Authorized Signature)
Title: _____	Title: _____	Title: _____	Title: _____	Title: _____	Title: _____
Date: _____	Date: _____	Date: _____	Date: _____	Date: _____	Date: _____

Approved by Funding Agency (if applicable)

By: \_\_\_\_\_ Date: \_\_\_\_\_  
Title: \_\_\_\_\_

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**CITY OF PORT ST. JOE PLANNING DEPARTMENT  
DEVELOPMENT ORDER APPLICATION PACKET**

**INCOMPLETE SUBMITTALS WILL NOT BE REVIEWED**

(The Building Department requires separate forms and fees to obtain building permits.)

NOTE: THE ADDRESS OF THE PROPERTY MUST BE POSTED PRIOR TO SUBMITTAL.

1. X Two complete sets of plans, drawn to scale.  
Including: A site plan with square feet of living, total square feet, impervious surface, and setbacks.

\*\* Setbacks are measured from the closest overhang to property line\*\*

A site plan showing any protected trees which will be removed from the property.  
(Protected trees are any trees other than pine larger than 8" in diameter measure 54" from the base of the tree.)

2. X Development Order Packet
3. \_\_\_\_\_ New Address application
4. \_\_\_\_\_ Complete City water meter impact form
5. \_\_\_\_\_ Complete Driveway permit application

*(Please refer to City of Port St. Joe's Land Development Regulations)*

**DESCRIPTION**

Project Address Cecil G Costin Sr. Blvd

Lot Square Footage: 76,838.92 SqFt Dwelling Square Footage: 20,655.00 SqFt

Driveway Square Footage: 510 Sqft (Garrison Ave) Accessory Building Square Footage: N/A

Pool Square Footage: N/A Patio/Deck Square Footage: N/A

Setbacks: Front: 30.0' Left Side: 25.0' Wetland Setback

Rear: 10.0' Right Side: 20.0'

Floor Area Ratio: 26.88% Lot Coverage: 64,261.06 SqFt

Building Height in Feet: 25.0' Impervious Surface: 52,387.63 SqFt

Landscape Buffers: (height x width) N/A Elevation: FFE 13.82'

Grant Ash  
Applicant Name

117 Sailors Cove Dr  
Applicant Address

850-340-1270  
Phone Number

[Signature]  
Applicant Signature

Oct 25, 2022  
Date



**RISH FAMILY PLAZA  
PORT ST. JOE  
GULF COUNTY, FLORIDA**

L. Jack Husband III, P.E.  
Florida Registration No. 69169  
Date: October 05, 2021

A handwritten signature in black ink, appearing to read "L. Jack Husband III", is written over a horizontal line.

120 N. HWY 71  
P. O. BOX 141  
WEWAHITCHKA, FL 32465  
(850) 639-3860  
WWW.SOUTHEASTERNGE.COM

115A SAILORS COVE DR  
P. O. BOX 951  
PORT ST. JOE, FL 32457  
(850) 227-1297  
INFO@SOUTHEASTERNGE.COM



**RISH FAMILY PLAZA  
PORT ST. JOE  
GULF COUNTY, FLORIDA**

**PROJECT DESCRIPTION**

Rish Family Plaza LLC is proposing to construct a new commercial development consisting of a 9 Unit Retail/Office Space on 11 existing parcels located in Section 1 Township 9S Range 11W, of Gulf County, Florida. The proposed development is more specifically located south of the intersection of Cecil G. Costin Sr. Blvd (FL-71) and Garrison Avenue on the north side of FL-71. The lots in which the development is proposed (Gulf County Property Appraiser Numbers: 04830-006R, 04830-007R, 04830-008R, 04830-009R, 04830-010R, 04830-011R, 04830-012R, 04830-013R, 04830-014R, 04830-015R and 04830-016R) for a total of approximately 76,838.92 SqFt (1.76 Acres). The eastern side of the property borders a 20' wide alley (that is not open to the public and has never been developed). The south side of the property borders Cecil B. Costin Blvd. (FL-71). The west side of the property borders Garrison Ave. The north side of the property borders a 20' wide alley (that is not open to the public and has never been developed). The proposed development will consist of a new commercial building designed for Retail/ Office Space.

The development will have a 24' wide asphalt drive aisle with a 6" modified FDOT Type "D" curb along edges of pavement. New drainage inlets and stormwater facilities will be installed throughout the site to capture and provide adequate stormwater treatment. The site will have three new storm water management facilities ranging from 1' deep to 3.5' deep with 3:1 side slopes.

The development will also provide a new sidewalk along Cecil G Costin Blvd. (FL-71) connecting to an existing sidewalk stub out at the intersection of Cecil G. Costin Blvd. and Garrison Ave.

To service the potable water needs of development the developer will install 9 new 3/4" water service connections with meters and backflows tapping into an existing 6" water main running parallel to FL-71 on the same side as the development. The developer will also provide a 6" tap on the same existing 6" water main to service a new fire hydrant located on the site. The developer will also provide a 1" tap on the off the existing 6" main for irrigation of the site.

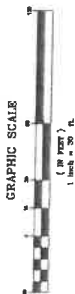
To service the sanitary sewer needs each unit will receive a 4" PVC sanitary lateral. That will connect to existing gravity sewer main running through the median of Cecil G. Costin Blvd. (FL-71).

For additional information please see the attached permit drawings.



Lots 15, 17, 19, 21, 23, 25, 27, 29, 31, 33, & 35, Block "28",  
Official Map of Port St. Joe, Florida, a subdivision on per  
map or plat thereof recorded in Plat Book 1, Page 17 in the  
Public Records of Gulf County, Florida less & except the southerly 30'

1. No improvements were located in this survey other than those shown herein.
2. No underground encroachments, utilities or foundations were located in this survey.
3. All measurements shown herein are Standard U.S. Survey Feet, and decimals thereof.
4. As recorded from Florida Hurricane Bole for Fort Collier, Florida, Community-Parcel Number 190456 0333 at Date of Map Issued March 8, 2021. This property is located in Zone "A", at Zone "A" elevation 9.0'.
5. The use of this survey is limited to the specific transaction herein.
6. Subject to zoning setbacks, easements and restrictions of record.
7. Adjacent deeds of record were not provided to this firm.  
 FDOT State Road 871 right of way shown herein is as depicted on FDOT right of way map Station 31020-2504, sheet 2 of 4, dated 5/17/1988.  
 Easements shown herein based on MAND 88 D07mm.



1228' R/W (OPEN/PAVED)  
STATE ROAD #71  
COSTIN SR  
BOULEVARD

SURVEYORS • MAPPERS

(850) 976-3010 FAX (850) 978-1110  
O Box 625 2813 Crossville Hwy Crossville, TN 38328

Not valid without the signature and the original raised seal of a Florida licensed surveyor and mapper.

WADE G. BROWN  
Surveyor & Mapper  
Florida Certificate No. 5959  
(LB# 6475)

THIS SURVEY IS CERTIFIED TO:  
 WILSON FAMILY PLAZA, LLC, a Florida limited liability company

CHARLES A. WHITEHEAD FOUNDATION;  
WATERSOUND TITLE AGENCY;  
FIDELITY NATIONAL TITLE INSURANCE COMPANY;

THIS SURVEY WAS PREPARED FOR THE SOLE BENEFIT OF THE ABOVE CERTIFIED ENTITIES AND/OR INDIVIDUALS AND IS INTENDED TO BE USED FOR THE CURRENT TRANSACTION ONLY. INDIVIDUALS OR ENTITIES WHICH ARE NOT SPECIFICALLY LISTED ABOVE ARE NOT ENTITLED TO RELY UPON THIS BOUNDARY SURVEY FOR ANY PURPOSE. FURTHERMORE, THIS SURVEYOR IS NOT OBLIGATED TO AND WILL NOT SUPPORT THIS BOUNDARY SURVEY TO ANY INDIVIDUAL OR ENTITY WHICH IS NOT SPECIFICALLY LISTED ABOVE.

REVISION	BY	DATE
ADDED TOPOGRAPHIC SURVEY	AA	3/21/022
REVISED LEGAL & CERTIFICATIONS		

## PREPARED BY:

EDWIN G. BROWN AND ASSOCIATES, INC.

OTWICK, PAUL	COUNTRY: GULF	TRAIN FR. EN	ENTERED FR. TGB
OTWICK, PAUL	SECTOR: 1	SCALE: 1" = 30'	ENTERED FR. TGB
OTWICK, PAUL	TRAIN: 8-THRU	SURVEY DATE:	NOVEMBER 14, 2004
OTWICK, PAUL	PLANT: 11-WEST	JOB NUMBER:	114
PLANT: 1 OF 1			
		21-746	45367

DOT - FLORIDA DEPARTMENT OF TRANSPORTATION  
DEP - FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION

### SUPPLEMENTARY NOTES

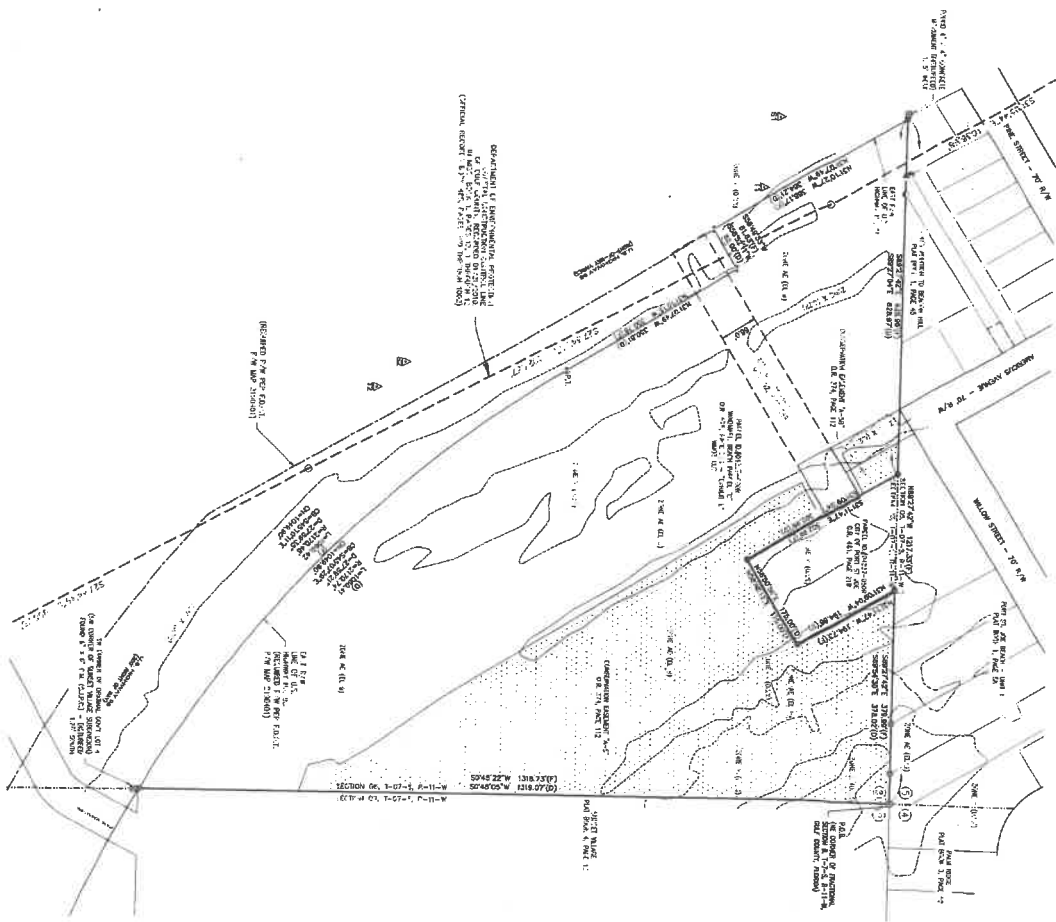
perceptions of the role of the police in the community. The study was conducted in a small town in the north of England, where the police have been perceived as being particularly effective in their role. The study was conducted in a small town in the north of England, where the police have been perceived as being particularly effective in their role. The study was conducted in a small town in the north of England, where the police have been perceived as being particularly effective in their role.

#### SOURCE CONTROL POINTS

- [illegible]

DINTs

**2.2.2. *Cellular fractionation and immunoblotting***—Cells were grown in 250 ml serum-containing medium in 75 cm<sup>2</sup> flasks (Corning Costar, High Wycombe, UK) and harvested by trypsinization. Cells were washed with ice-cold PBS and then harvested by centrifugation at 400g for 5 min. Cells were washed again with ice-cold PBS and then resuspended in 1 ml of lysis buffer (1% NP-40, 0.5% deoxycholate, 0.5% sodium dodecyl sulfate, 1% Triton X-100, 150 mM NaCl, 50 mM Tris-HCl, pH 7.4, 50 mM NaF, 50 mM NaPP<sub>6</sub>, 50 mM NaVO<sub>3</sub>, 50 mM NaP<sub>2</sub>O<sub>7</sub>, 1 mM EDTA, 1 mM EGTA, 1 mM DTT, 1 mM PMSF, 1 mM benzamide hydrochloride, 1 mM Na<sub>2</sub>MoO<sub>4</sub>·2H<sub>2</sub>O, 1 mM Na<sub>2</sub>WO<sub>4</sub>·2H<sub>2</sub>O, 1 mM Na<sub>2</sub>SeO<sub>3</sub>·10H<sub>2</sub>O, 1 mM Na<sub>2</sub>SO<sub>3</sub>·10H<sub>2</sub>O, 1 mM Na<sub>2</sub>CO<sub>3</sub>·10H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·12H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·7H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·6H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·3H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·2H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.5H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.25H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.03125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.015625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0078125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00390625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.001953125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0009765625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00048828125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000244140625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0001220703125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00006103515625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000030517578125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0000152587890625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00000762939453125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000003814697265625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0000019073486328125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00000095367431640625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000000476837158203125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0000002384185791015625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00000011920928955078125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000000059604644775390625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0000000298023223876953125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00000001490116119384765625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000000007450580596923828125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.0000000037252902984619140625H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.00000000186264514923095703125H<sub>2</sub>O, 1 mM Na<sub>2</sub>HPO<sub>4</sub>·0.000000000931322574615478515625H<sub>2</sub>O, 1 mM 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## SYMBOLS &amp; ABBREVIATIONS:

- L.B. = LETTERED PROJECT;  
 S.B. = SYMBOLIC PROJECT;  
 U.B. = UNKNOWN;  
 1. = ONE; 2. = TWO;  
 3. = THREE; 4. = FOUR;  
 5. = FIVE; 6. = SIX;  
 7. = SEVEN; 8. = EIGHT;  
 9. = NINE;  
 10. = TEN;  
 11. = ELEVEN;  
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 13. = THIRTEEN;  
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INDEX OF SHEETS:

21. LIFE DRAINAGE DETAIL, 4 ZONE POND,  
100 W.C. INLET, 410 LTRON.

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**Dewberry**  
DEWBERRY ENGINEERS INC.  
203 ABERDEEN PARKWAY  
PANAMA CITY, FLORIDA 32405  
PHONE: 850.522.0644 FAX: 850.522.1011  
WWW.DEWBERRY.COM  
CERTIFICATE OF AUTHORIZATION NO. LB 8015

REVISIONS		
DATE	REVISION	CHK
10/24/2022	ADDED ADDITIONAL TOPO OF U.S. 98 FOR F.D.O.T. PERMIT	DB

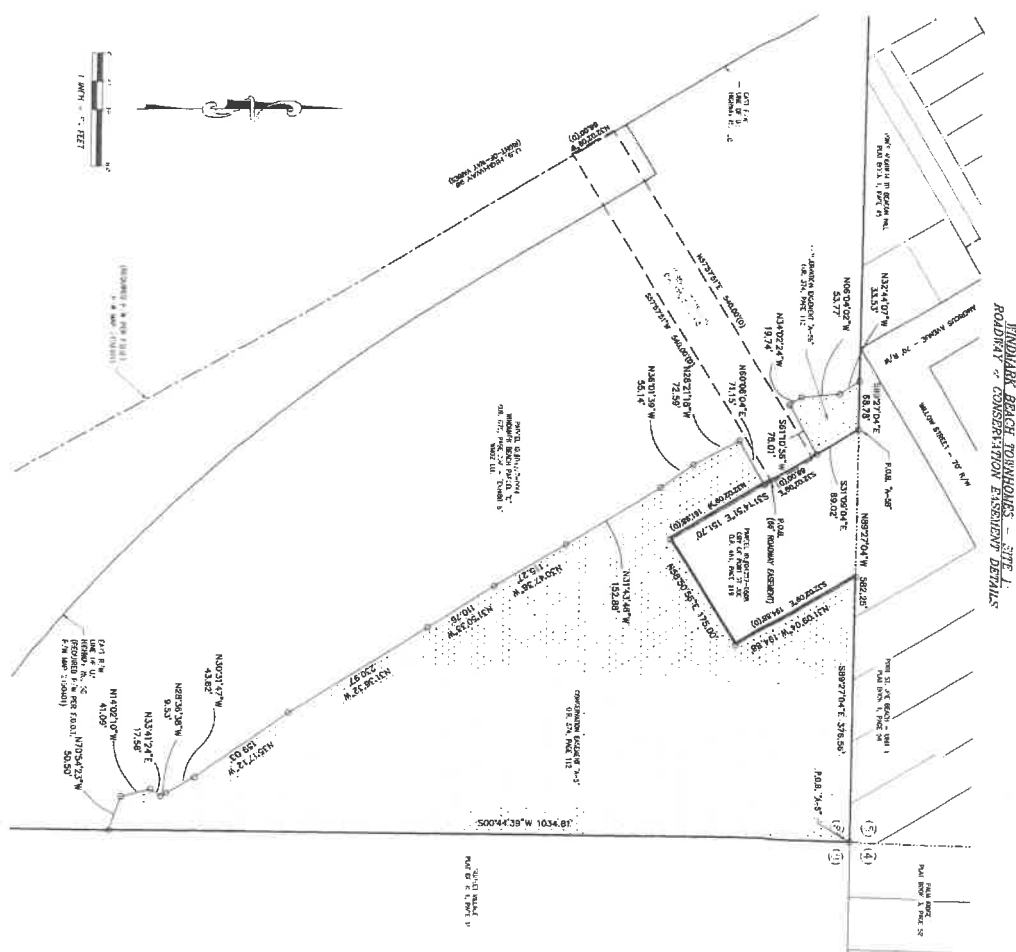
BOUNDARY & TOPOGRAPHIC SURVEY  
SECTION 08, TOWNSHIP 07  
SOUTH, RANGE 11 WEST  
WINDMARK NORTH TOWNHOMES  
GULF COUNTY, FLORIDA

DAVID JON BARTLETT, P.S.M. DATE SIGNED  
PROFESSIONAL LAND SURVEYOR No. 154018

PROJECT NO.  
50144759  
SCALE  
1" = 100'

51

October 24, 2022 (10:48:11 EST)  
DRAWING NAME: M:\00144739\_5



INDEX OF SHEETS.

1. LITE QUANTITIES, DETAILS, CEMENT PLASTER, ROOFING, STAIRS, AND ELEVATIONS.

2. THE EXISTENT DETAIL.

3. THE PROPOSED ROOFING DETAIL.

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BOUNDARY & TOPOGRAPHIC SURVEY  
SECTION 08, TOWNSHIP 07  
SOUTH, RANGE 11 WEST  
WINDMARK BEACH TOWNHOMES  
GULF COUNTY, FLORIDA

DANIEL JON BARTLEY, P.E.      DAVE SOMER  
PROFESSIONAL LAND SURVEYOR No. 154088



REVISIONS		
DATE	REVISION	OK'D
10/24/2022	ADDED ADDITIONAL TOPD OF U.S. \$6 FOR F.D.O.T. PERMIT	DB









## FLORIDA DEPARTMENT OF Environmental Protection

Northwest District  
160 W. Government Street, Suite 308  
Pensacola, Florida 32502-5740  
NWD\_EPOST@dep.state.fl.us

**Ron DeSantis**  
Governor

**Jeanette Nuñez**  
Lt. Governor

**Shawn Hamilton**  
Secretary

January 12, 2023

BY ELECTRONIC MAIL  
[grish@rishreg.com](mailto:grish@rishreg.com)

### NOTIFICATION OF ACCEPTANCE OF USE OF A GENERAL PERMIT

**PERMITTEE:**  
Rish Family Market, LLC  
By: Grant Rish, Owner  
117 Sailors Cove  
Port St. Joe, FL 32456

<b>PERMIT NUMBER:</b>	0409416-003-DWC/CG
<b>ISSUE DATE:</b>	January 12, 2023
<b>EXPIRATION DATE:</b>	January 11, 2028
<b>COUNTY:</b>	Gulf
<b>PROJECT NAME:</b>	Rish Family Market
<b>WASTEWATER TREATMENT:</b>	City of Port St Joe WWTF
<b>FACILITY ID:</b>	FLA020206

Dear Mr. Rish:

This letter acknowledges receipt of your *Notification/Application for Constructing a Domestic Wastewater Collection/Transmission System* for the Rish Family Market sewer project, 0409416-003-DWC/CG. Our Office received the Notice on January 9, 2023.

This letter is to advise you that the Department does not object to your use of such general permit.

The domestic wastewater collection/transmission system project consists of the construction of 309 linear feet of 8" gravity sewer main with 3 manholes and a duplex grinder lift station with approximately 4,551 LF of 4-inch HDPE force main for service to the Rish Family Market commercial development. The proposed lift station's 4-inch force main will connect to the existing 4-inch force main located at the intersection of Jones Homestead Road and SR-30.

The wastewater from the newly constructed sewer system will eventually flow to the City of Port St Joe WWTF, FLA020206, for treatment.

The project development site is located at the intersection of SR-30 (US Hwy 98) and SR-30A in Port St. Joe, Gulf County, Florida.

The construction shall be in accordance with the construction drawings certified by Tyler Marsh, P.E., on November 30, 2022.

PROJECT NAME: Rish Family Market  
PERMIT NUMBER: 0409416-003-DWC/CG  
NOTIFICATION OF ACCEPTANCE OF USE OF A GENERAL PERMIT  
Page 2 of 5

Please note the attached requirements apply to your use of this general permit for constructing the proposed domestic wastewater collection/transmission system.

You are further advised that the construction activity must conform to the description contained in your *Notification/Application for Constructing a Domestic Wastewater Collection/Transmission System* and that any deviation will subject the permittee to enforcement action and possible penalties.

When referring to this project, please use the project name and file number indicated above. If you have any questions, contact Chuck Harrell by email at [charles.harrell@FloridaDEP.gov](mailto:charles.harrell@FloridaDEP.gov) or by phone at (850) 595-0581.

Sincerely,



Katie Ates, P.E.  
Water and Wastewater Permitting Manager

c:

Tyler Marsh, P.E., Southeastern Consulting Engineers, Inc  
([tyler.marsh@southeasternce.com](mailto:tyler.marsh@southeasternce.com))

Kevin Pettis, Plant Manager, City of Port St. Joe ([kpettis@psj.fl.gov](mailto:kpettis@psj.fl.gov))

John Grantland, Public Works Director, City of Port St. Joe ([jgrantland@psj.fl.gov](mailto:jgrantland@psj.fl.gov))

Michael Hammond, Gulf County Administrator ([bocc@gulfcounty-fl.gov](mailto:bocc@gulfcounty-fl.gov))

Florida Department of Health Gulf County, Environmental Supervisor  
([gulfchd@flhealth.gov](mailto:gulfchd@flhealth.gov))

**REQUIREMENTS FOR USE OF THE GENERAL PERMIT FOR DOMESTIC WASTEWATER COLLECTION/TRANSMISSION SYSTEMS:**

1. This general permit is subject to the general permit conditions of Rule 62-4.540, F.A.C., as applicable. This rule is available at the Department's Internet site at: <http://www.dep.state.fl.us/water/rulesprog.htm#ww> [62-4.540]
2. This general permit does not relieve the permittee of the responsibility for obtaining a dredge and fill permit where it is required. [62-604.600(6)(b)1]
3. This general permit cannot be revised, except to transfer the permit. [62-604.600(6)(b)2]
4. This general permit will expire five years from the date of issuance. If the project has been started and not completed by that time, a new permit must be obtained before the expiration date in order to continue work on the project. [62-4.030]
5. Upon completion of construction of the collection/transmission system project, and before placing the facilities into operation for any purpose other than testing for leaks or testing equipment operation, the permittee shall submit to the Department's District Office Form 62-604.300(8)(b), Notification of Completion of Construction for a Domestic Wastewater Collection/Transmission System. This form is available at the Department's Internet site at: <https://floridadep.gov/water/domestic-wastewater/content/domestic-wastewater-forms> instructions for submitting forms electronically are available at DEP's wastewater forms webpage or submittal may be made using the DEP Business Portal <https://www.fldepportal.com/DepPortal/go/home> by selecting "Submit," "Registration/Notification," "Submit Notifications to DEP," then choose submission type "Division of Water Resource Management Domestic/Industrial Wastewater" and "Notification of Completion of Construction for a Domestic Wastewater Collection/Transmission System." [62-604.700(2)]
6. The new or modified collection/transmission facilities shall not be placed into service until the Department clears the project for use. [62-604.700(3)]
7. Abnormal events shall be reported to the Department's Northwest District Office in accordance with Rule 62-604.550, F.A.C. For unauthorized spills of wastewater in excess of 1000 gallons per incident, or where information indicates that public health or the environment may be endangered, oral reports shall be provided to the STATE WATCH OFFICE TOLL FREE NUMBER (800)320-0519 as soon as practical, but no later than 24 hours from the time the permittee or other designee becomes aware of the circumstances. Unauthorized releases or spills less than 1000 gallons per incident are to be reported orally to the Department's Northwest District Office within 24 hours from the time the permittee, or other designee becomes aware of the circumstances. [62-604.550]

The oral notification shall be followed by a written submission, which shall be provided within five days of the time that the owner/operator becomes aware of the circumstances. The written submission shall contain: a description of the spill, release or abnormal event and its cause; the period and duration of noncompliance including exact dates and times, and if the noncompliance has not been corrected, the anticipated time it is expected to continue; clean-up actions taken and status; steps taken or planned to reduce, eliminate, and prevent recurrence; the type of sanitary sewer overflow structure (e.g., manhole); the discharge location address and latitude/longitude; type of water discharged; discharge



volumes and volumes recovered; volume discharged to surface waters and receiving waterbody name; types of human health and environmental impacts of the sanitary sewer overflow (e.g., beach closure); whether the noncompliance was caused by a third party (e.g., contractor); and, whether the sanitary sewer overflow was related to wet weather. The written submission shall be provided electronically. Electronic submission is available using the Department's Business Portal at <https://www.fldepportal.com/go/> (via "Submit" followed by "Report" or "Registration/Notification").

8. In accordance with Section 403.077, F.S., unauthorized releases or spills reportable to the State Watch Office pursuant to paragraph 7 above shall also be reported to the Department within 24 hours from the time the permittee becomes aware of the discharge. The permittee shall provide to the Department information reported to the State Watch Office. Notice of unauthorized releases or spills may be provided to the Department through the Department's Public Notice of Pollution web page at <https://floridadep.gov/pollutionnotice> or by reporting electronically using the Department's Business Portal at <https://www.fldepportal.com/go/> (via "Submit" followed by "Report" or "Registration/Notification").

#### **ADDITIONAL INFORMATION:**

Once a collection/transmission system is cleared for operation, the provisions below shall be met by the owner/operator of the system in accordance with Rule 62-604.500, F.A.C.

1. All collection/transmission systems shall be operated and maintained to provide uninterrupted service. All pump stations shall be operated and maintained to provide the emergency pumping capability requirements in paragraph 62-604.400(2)(a), F.A.C., the lightning and transient voltage surge protections in paragraph 62-604.400(2)(b), F.A.C., and the design and signage requirements in paragraph 62-604.400(2)(d), F.A.C. Also, all equipment, pipes, manholes, pump stations, and other appurtenances necessary for the collection/transmission of domestic wastewater, including equipment provided pursuant to subsection 62-604.400(2), F.A.C., shall be maintained to function as intended. [62-604.500(2) and (3)]
2. The owner/operator of a collection/transmission system shall evaluate and update the emergency response plan portion of the operation and maintenance manual annually. The emergency response plan shall assess system security including cybersecurity; water quality monitoring for sanitary sewer overflows affecting surface waters; and, hurricane and severe storm preparedness and response. [62-604.500(4)]
3. Collection/transmission systems shall be maintained to minimize excessive infiltration and inflow into the collection/transmission system, as well as excessive leakage from the collection/transmission system. The owner/operator of a collection/transmission system shall take corrective actions when infiltration, inflow, or leakage is excessive. Infiltration and inflow are considered excessive if one or both cause or contribute to sanitary sewer overflows. Leakage, or exfiltration, is considered excessive if it causes or contributes to a violation of surface water quality standards or ground water quality standards. [62-604.500(5)]

4. All collection/transmission systems shall be operated and maintained to prevent sanitary sewer overflows. Owners/operators shall evaluate the cause of all sanitary sewer overflows and evaluate potential corrective measures to avoid future sanitary sewer overflows. Corrective actions shall be taken by the owner/operator of the collection/transmission system if excessive inflow and infiltration causes or contributes to a sanitary sewer overflow. The owner/operator of a satellite collection system shall take corrective actions for a sanitary sewer overflow in the receiving collection system caused by excessive inflow and infiltration in the satellite collection system. [62-604.500(6)]
5. The approved Operation and Maintenance Manual and emergency response plan pursuant to Rule 62-604.500(4), F.A.C., shall be kept available at a site convenient for use by operation and maintenance personnel and for inspection by the Florida Department of Environmental Protection personnel. [62-604.500(4)]



Dewberry Engineers Inc.  
203 Aberdeen Parkway  
Panama City, FL 32405

850.522.0644  
850.522.1011 fax  
www.dewberry.com

October 25, 2022

**VIA Hand Delivery**

Mr. Jim Anderson  
City Manager  
City of Port St. Joe  
P.O. Box 278  
Port St. Joe FL 32457

**RE: Windmark Beach North Townhomes  
Development Order Submittal  
Dewberry Project No. 5D144759**

Dear Mr. Anderson:

On behalf of The St. Joe Company, Dewberry is pleased to submit this Development Order submittal package to The City of Port St. Joe for the *Windmark Beach North Townhomes* project.

The Windmark Beach North Townhomes project is the construction of a new 78 lot townhome subdivision with supporting roadways, a stormwater management facility, and utilities in Port. St. Joe, FL. The project is east of US Highway 98 and adjacent to the Windmark Beach developments. Access to the site is via US Highway 98.

There will be one (1) stormwater management facility within this project, and it will discharge to unnamed wetlands. The proposed stormwater management facility has been designed to treat and attenuate onsite stormwater runoff to ERP and The City of Port St. Joe requirements.

Please find attached the following documents for your review:

- Development Order Application
- Development Order Fee - \$3,000 Check
- Two (2) S/S Windmark North Townhomes Plan Sets
- Two (2) S/S Windmark North Townhomes Stormwater Management Reports
- Two (2) S/S Lift Station Reports
- E-911 Addressing Application

Additionally, the FDEP Potable Water and Domestic Wastewater permit applications are enclosed for city review and signature. Please notify us if there are any concerns or when the applications are ready for pick-up.

Please review the enclosed materials. If you have any questions or need additional information, please contact us at (850) 571-1199 or you may e-mail me at [cshortt@dewberry.com](mailto:cshortt@dewberry.com).

Sincerely,  
DEWBERRY



Christopher Shortt, P.E.  
Project Manager

K:\50144759\_SJC\_Windmark N. Townhomes\Permits\PSJ DO\2022-10-24 DO Submittal

**CITY OF PORT ST. JOE PLANNING DEPARTMENT  
DEVELOPMENT ORDER APPLICATION PACKET**

**INCOMPLETE SUBMITTALS WILL NOT BE REVIEWED**

(The Building Department requires separate forms and fees to obtain building permits.)

NOTE: THE ADDRESS OF THE PROPERTY MUST BE POSTED PRIOR TO SUBMITTAL.

1. \_\_\_\_\_ Two complete sets of plans, drawn to scale.  
Including: A site plan with square feet of living, total square feet, impervious surface, and setbacks.

\*\* Setbacks are measured from the closest overhang to property line\*\*

A site plan showing any protected trees which will be removed from the property.  
(Protected trees are any trees other than pine larger than 8" in diameter measure 54" from the base of the tree.)

2. \_\_\_\_\_ Development Order Packet  
3. \_\_\_\_\_ New Address application  
4. \_\_\_\_\_ Complete City water meter impact form  
5. \_\_\_\_\_ Complete Driveway permit application

**(Please refer to City of Port St. Joe's Land Development Regulations)**

DESCRIPTION Windmark North Townhomes

Project Address US HWY 98

Lot Square Footage: 1860-3700 sq. ft. Dwelling Square Footage: Varies  
(0.043-0.085 acres)

Driveway Square Footage: Varies Accessory Building Square Footage: N/A

Pool Square Footage: N/A Patio/Deck Square Footage: N/A

Setbacks: Front: 10' Left Side: 5'

Rear: 5' Right Side: 5'

Floor Area Ratio: VARIES Lot Coverage: VARIES

Building Height in Feet: LESS THAN 35' Impervious Surface: Max 75%

Landscape Buffers: (height x width) Varies Elevation: Varies

Jason Scarbrough  
Applicant Name

130 N Richard Jackson Blvd, Suite  
200, Panama City Beach, FL 32407 (850)-231-6530  
Applicant Address Phone Number

\*  Applicant Signature

\* 9.15.22  
Date



**GULF COUNTY E-911**  
**Official House Number Form**  
1000 Cecil G Costin Sr Blvd Bldg. 500  
Port St. Joe, FL 32456

Email: e911@gulfcountry-fl.gov

[http://www.gulfcountry-fl.gov/counv\\_government/e911](http://www.gulfcountry-fl.gov/counv_government/e911)

Date: 06/30/2022

Voice: (850) 229-9111

Fax: (850) 665-3427

**REQUESTING A NEW ADDRESS AND VERIFICATION**

- 1) Parcel Number: 04227-000R (ex. 99999-999R)  
Street Name: \_\_\_\_\_  
Location: (Please circle one) Port St Joe Wewahitchka Gulf County (Unincorporated Area)  
2) Has this parcel ever had a building with or without an address on it before today? Yes No  
If yes, what is/was that address? \_\_\_\_\_  
3) Type of Structure to be built or brought in: (check all boxes that apply)  
\_\_\_\_ Commercial-What Type? \_\_\_\_\_ RV \_\_\_\_\_ Utility  
\_\_\_\_ Single Family \_\_\_\_\_ Mobile Home  
\_\_\_\_ Service Upgrade \_\_\_\_\_ X \_\_\_\_\_ Other Please Explain New residential townhome development (Windmark Beach)  
4) Name of Property Owner: St. Joe Company  
5) Telephone: ( ) \_\_\_\_\_  
6) Name of person requesting information if different than above: Jason Scarbrough  
7) Telephone: (850) 231-6536 9) Other helpful info: \_\_\_\_\_  
8) Fax: ( ) \_\_\_\_\_ or Email: Jason.scarbrough@joe.com

**After completing the above information please follow the steps below.**

- A) Take or fax this form to Gulf County E-911 at 850-229-9115 to receive your official address  
B) Take completed form back to the Building Department to start the permitting process.  
C) \*MOST IMPORTANTLY! After receiving your address, PLEASE display it on the construction site and/or house when completed where it will be easily seen from the road.

**E-911 DEPARTMENT (located in the EOC)**

\*This Box for Official Use Only\*

Structure Type Being Addressed: \_\_\_\_\_

Address: \_\_\_\_\_ Map # \_\_\_\_\_

Approved By: \_\_\_\_\_ Date: \_\_\_\_\_

**COMMENTS:**

**SITE PLAN:**

☒ Check box if site plan attached

**Disclaimer-** If Gulf County E-911 finds any information supplied to be incorrect or has changed, this address could become void and a new one will be issued so it meets the Enhanced 911 rules and regulations.

2018



## Florida Department of Environmental Protection

### Notification/Application for Constructing a Domestic Wastewater Collection/Transmissions System

#### Part I - General

##### Subpart A: Permit Application Type (Check only one)\*

- ☐ Individual permit for a domestic wastewater collection/transmission system serving **10 or greater** equivalent dwelling units (EDU). An EDU is equal to 3.5 persons. Criteria for an individual permit are contained in Rule 62-604.600(7), F.A.C.  
Application fee: \$500
- ☐ Individual permit for a domestic wastewater collection/transmission system serving **less than 10** equivalent dwelling units (EDU). An EDU is equal to 3.5 persons. Criteria for an individual permit are contained in Rule 62-604.600(7), F.A.C.  
Application fee: \$300
- ☐ Minor revision to an individual permit for a domestic wastewater collection/transmission system.  
Application fee: \$250
- ☒ Notice of Intent to use the general permit for a domestic wastewater collection/transmission system. Criteria for a general permit are contained in Rule 62-604.600(6), F.A.C. Projects not meeting the criteria in Rule 62-604.600(6), F.A.C., must apply for an individual permit.  
Application fee: \$250

\*Note: Each non-contiguous project (i.e., projects that are not interconnected or are not located on adjacent streets or in the same neighborhood) requires a separate application and fee.

##### Subpart B: Instructions

- (1) This form shall be completed for all public and private domestic wastewater collection/transmission system construction projects as follows:
  - If this is a Notice of Intent to use the general permit, this notification shall be submitted to the Department at least **30 days** prior to initiating construction.
  - If this is an application for an individual permit, the permit must be obtained prior to initiating construction.
- (2) One copy of the completed form shall be submitted to the appropriate DEP district office or delegated local program along with the appropriate fee, and one copy of the following supporting documents. Checks should be made payable to the Florida Department of Environmental Protection, or the name of the appropriate delegated local program. Forms and documents may be submitted electronically in accordance with the [Wastewater Electronic Document Submission](#) instructions available from DEP's website.
  - If this is a Notice of Intent to use the general permit, attach a site plan or sketch showing the size and approximate location of new or altered gravity sewers, pump stations and force mains; showing the approximate location of manholes and isolation valves; and showing how the proposed project ties into the existing or proposed wastewater facilities. The site plan or sketch shall be signed and sealed by a professional engineer registered in Florida.
  - If this is an application for an individual permit, one set of plans and specifications shall be submitted with this application. The plans and specifications shall include lift station design calculations if a lift station is proposed. Chapters 10 and 20 of *Recommended Standards for Wastewater Facilities*, 2014, provide helpful guidance on the proper preparation of plans and specifications. The plans and specifications shall be signed and sealed by a Professional Engineer registered in Florida.
- (3) All information shall be typed or printed in ink if submitting paper forms. Where attached sheets (or other technical documentation) are utilized in lieu of the blank spaces provided, indicate appropriate cross-references on the form. For Items (1) through (4) of Part II of this application form, if an item is not applicable to your project, indicate "NA" in the appropriate space provided.



## Part II – Project Documentation

### (1) Collection/Transmission System Permittee

Name Jason Scarbrough Title Project Manager  
 Company Name The St. Joe Company  
 Address 130 N Richard Jackson Blvd., Suite 200  
 City Panama City Beach State FL Zip 32407  
 Telephone 850-231-6530 Cell \_\_\_\_\_ Fax \_\_\_\_\_  
 Email jason.scarbrough@joe.com

### (2) General Project Information

Project Name Windmark Beach North Townhomes  
 Project Address \_\_\_\_\_  
 City Port St. Joe State Florida Zip 32456  
 County Gulf Latitude \_\_\_\_\_ Longitude \_\_\_\_\_

Project Description and Purpose (including the total length and material of each diameter of proposed gravity sewers and forcemains, total number of manholes, total number of pump stations, and connections to existing system):

Windmark Beach North Townhomes will include 1 duplex lift station, 1,094 LF of 8" gravity sewer, 5 sanitary sewer manholes, and 1,464 LF of 4" Force Main.

Estimated date for: Start of construction May 2023 Completion of Construction December 2023

Number of connections to existing system or treatment plant 78 Townhomes

### (3) Project Capacity

Type of Unit	Number of Units	Population Per Unit	Total Population (Number of Units x Population Per Unit)	Per Capita Flow in Gallons per Day (GPD)	Total Average Daily Flow in GPD (Total Population x Per Capita Flow)	Peak hour flow in Gallons Per Minute (GPM)
Single-Family Home	78	3	234	100	23400	67
Mobile Home						
Apartment						
Commercial, Institutional, or Industrial Facility*						
Total	NA	NA	1338	NA	23400	67

\* Description of commercial, institutional, and industrial facilities and explanation of method used to estimate per capita flow for these facilities:

PHF=4.12 from Ten State Standards  
 ADF=23400/24/60= 16.3 GPM  
 PHF=16.3x4.12=67 GPM

26

(4) Pump Station Data (attached additional sheets as necessary)

Location	Type	Maximum Estimated Flow to the Station (GPD)	Average Estimated Flow to the Station (GPD)	Minimum Estimated Flow to the Station (GPD)	Operating Conditions [GPM @ FT (TDH)]
Shown on Plans	Lift Station	96,408	23,400	5,679.6	10-245@92-98'

(5) Collection/Transmission System Design Information

- A. This information must be completed for all projects by the applicant's professional engineer, and if applicable, those professional engineers in other disciplines who assisted with the design of the project. The checklist below shall be used for conventional collection/transmission systems while Attachment I to this form shall be used for low pressure sewer systems, including septic tank effluent pump (STEP) systems, and Attachment II shall be used for vacuum sewer systems (include Attachments I or II with the submittal of this form as applicable). These checklists cover important items but are not necessarily completely comprehensive of collection system construction and do not relieve the engineer from designing the collection system following sound engineering practices.

Complete the tables below (or Attachments I or II as applicable) as follows:

- The engineer shall initial each requirement if the project has been designed to comply with the standard or criteria.
- Mark "NA" if the requirement does not apply to this project and provide an explanation in section (5)B.
- Mark "NC" if the project has not been designed to comply with the requirement and provide an explanation in section (5)B, including any rule references.

Note, if the project has not been designed in accordance with the standards and criteria set forth in Rules 62-604.400(1) and (2), F.A.C., an application for an individual permit shall be submitted. However, if Rules 62-604.400(1) and (2), F.A.C., specifically allow for another alternative that will result in an equivalent level of reliability and public health protection, the project can be constructed using the general permit. Also note that each requirement below and in Attachments I and II includes a reference to guidance or rule for further information.

The guidance documents given in the checklists are as follows:

- "RSWF" – Recommended Standards for Wastewater Facilities (2014). Health Research, Inc., Health Education Services Division, P.O. Box 7126, Albany, NY 12224, [www.healthresearch.org](http://www.healthresearch.org)
- "MOPFD-12" – Alternative Sewer Systems, Manual of Practice No. FD-12. Alternative Sewer Systems (1986). Water Environment Federation, 602 Wythe Street, Alexandria, VA 22314, [www.wef.org](http://www.wef.org).
- "FL DSG" – Design and Specification Guidelines for Low Pressure Sewer Systems (1981). Department of Environmental Protection, 2600 Blair Stone Road, MS 3540, Tallahassee, FL 32399-2400, [www.floridadep.gov](http://www.floridadep.gov).
- "EPA ACS" - Alternative Wastewater Collection Systems (1991). EPA/625/1-91/024. NTIS# PB93-1162591N2; National Technical Information Service, 5285 Port Royal Road, Springfield, VA 22161, [www.ntis.gov](http://www.ntis.gov).

General Requirements

Initials (or "NA" or "NC")	Item Number	Requirement
CS	1	The project is designed based on an average daily flow of 100 gallons per capita plus wastewater flow from industrial plants and major institutional and commercial facilities unless water use data or other justification is used to better estimate the flow. The design includes an appropriate peaking factor, which covers I/I contributions and non-wastewater connections to those service lines. (Note, see Attachment I for low pressure sewer systems) [RSWF 11.243]
CS	2	Procedures are specified for operation of the collection/transmission system during construction if work is performed on a system currently in operation. [RSWF20.15]
CS	3	The project is designed to be located on public rights-of-way, land owned by the permittee, or easements and to be located no closer than 100 feet from a public drinking water supply well and no closer than 75 feet from a private drinking water supply well; or documentation is provided in Part II.(5)B., showing that another alternative will result in an



Initials (or "NA" or "NC")	Item Number	Requirement
		equivalent level of reliability and public health protection. [62-604.400(1)(b) and (c), F.A.C.]
CS	4	The project is designed with no physical connections between a public or private potable water supply system and a sewer or force main and with no water pipes passing through or coming into contact with any part of a sewer manhole. [RSFW 38.1]
CS	5	The project is designed to preclude the deliberate introduction of storm water, surface water, groundwater, roof runoff, subsurface drainage, swimming pool drainage, air conditioning system condensate water, non-contact cooling water except as provided by Rule 62-610.668(1), F.A.C., and sources of uncontaminated wastewater, except to augment the supply of reclaimed water in accordance with Rule 62-610.472(3)(c), F.A.C. [62-604.400(1)(d), F.A.C.]
CS	6	The project is designed so that all new or relocated, buried sewers and force mains, are located in accordance with the separation requirements from water mains and reclaimed water lines of Rules 62-604.400(2)(g) and (h), F.A.C. Note, if the criteria of Rules 62-604.400(2)(g) 4. or (2)(h)3., F.A.C., are used, describe in Part II.(5)B. alternative construction features that will be provided to afford a similar level of reliability and public health protection. [62-604.400(2)(g) and (h), F.A.C.; 62-555.314, F.A.C.]

#### Gravity Sewers

Initials (or "NA" or "NC")	Item Number	Requirement
CS	7	The project is designed with no public gravity sewer conveying raw wastewater less than 8 inches in diameter. [RSWF 33.1]
CS	8	The design considers buoyancy of sewers, and appropriate construction techniques are specified to prevent flotation of the pipe where high groundwater conditions are anticipated. [RSWF 33.3]
CS	9	All sewers are designed with slopes to give mean velocities, when flowing full, of not less than 2.0 feet per second, based on Manning's formula using an "n" value of 0.013; or if it is not practicable to maintain these minimum slopes and the depth of flow will be 0.3 of the diameter or greater for design average flow, the owner of the system has been notified that additional sewer maintenance will be required. The pipe diameter and slope are selected to obtain the greatest practical velocities to minimize solids deposition problems. Oversized sewers are not specified to justify flatter slopes. [RSWF 33.41, 33.42, and 33.43]
CS	10	Sewers are designed with uniform slope between manholes. [RSWF 33.44]
NA	11	Where velocities greater than 10 fps are designed, provisions to protect against displacement by erosion and impact are specified. [RSWF 33.45]
NA	12	Sewers on 20% slopes or greater are designed to be anchored securely with concrete, or equal, anchors spaced as follows: not over 36 feet center to center on grades 20% and up to 35%; not over 24 feet center to center on grades 35% and up to 50%; and not over 16 feet center to center on grades 50% and over. [RSWF 33.46]
CS	13	Sewers 24 inches or less are designed with straight alignment between manholes. Where curvilinear sewers are proposed for sewers greater than 24 inches, the design specifies compression joints; ASTM or specific pipe manufacturer's maximum allowable pipe joint deflection limits are not exceeded; and curvilinear sewers are limited to simple curves which start and end at manholes. [RSWF 33.5]
N/A CS	14	Suitable couplings complying with ASTM specifications are required for joining dissimilar materials. [RSWF 33.7]
	15	Sewers are designed to prevent damage from superimposed loads. [RSWF 33.7]
CS	16	Appropriate specifications for the pipe and methods of bedding and backfilling are provided so as not to damage the pipe or its joints, impede cleaning operations and future tapping, nor create excessive side fill pressures and ovalation of the pipe, nor seriously impair flow capacity. [RSWF 33.81]
CS	17	Appropriate deflection tests are specified for all flexible pipe including PVC. Testing is

Initials (or "NA" or "NC")	Item Number	Requirement
		required after the final backfill has been in place at least 30 days to permit stabilization of the soil-pipe system. Testing requirements specify: 1) no pipe shall exceed a deflection of 5%; 2) using a rigid ball or mandrel for the deflection test with a diameter not less than 95% of the base inside diameter or average inside diameter of the pipe, depending on which is specified in the ASTM specification, including the appendix, to which the pipe is manufactured; and 3) performing the test without mechanical pulling devices. [RSWF 33.85]
CS	18	Leakage tests are specified requiring that: 1) the leakage exfiltration or infiltration does not exceed 100 gallons per inch of pipe diameter per mile per day for any section of the system; 2) exfiltration or infiltration tests be performed with a minimum positive head of 2 feet; and 3) air tests, as a minimum, conform to the test procedure described in ASTM C-828 for clay pipe, ASTM C 924 for concrete pipe, ASTM F-1417 for plastic pipe, and for other materials appropriate test procedures. [RSWF 33.93, 33.94, and 33.95]
NA	19	If an inverted siphon is proposed, documentation of its need is provided in Part II.(5)B. Inverted siphons are designed with: 1) at least two barrels; 2) a minimum pipe size of 6 inches; 3) necessary appurtenances for maintenance, convenient flushing, and cleaning equipment; and 4) inlet and discharge structures having adequate clearances for cleaning equipment, inspection, and flushing. Design provides sufficient head and appropriate pipe sizes to secure velocities of at least 3.0 fps for design average flows. The inlet and outlet are designed so that the design average flow may be diverted to one barrel, and that either barrel may be cut out of service for cleaning. [RSWF 35]

#### Manholes

Initials (or "NA" or "NC")	Item Number	Requirement
CS	20	The project is designed with manholes at the end of each line; at all changes in grade, size, or alignment; at all intersections; and at distances not greater than 400 feet for sewers 15 inches or less and 500 feet for sewers 18 inches to 30 inches, except in the case where adequate modern cleaning equipment is available at distances not greater than 600 feet. [RSWF 34.1]
CS	21	Design requires drop pipes to be provided for sewers entering manholes at elevations of 24 inches or more above the manhole invert. Where the difference in elevation between the incoming sewer and the manhole invert is less than 24 inches, the invert is designed with a fillet to prevent solids deposition. Inside drop connections (when necessary) are designed to be secured to the interior wall of the manhole and provide access for cleaning. Design requires the entire outside drop connection be encased in concrete. [RSWF 34.2]
CS	22	Manholes are designed with a minimum diameter of 48 inches and a minimum access diameter of 24 inches. [RSWF 34.3]
CS	23	Design requires that a bench be provided on each side of any manhole channel when the pipe diameter(s) are less than the manhole diameter and that no lateral sewer, service connection, or drop manhole pipe discharges onto the surface of the bench. [RSWF 34.5]
CS	24	Design requires: 1) manhole lift holes and grade adjustment rings be sealed with non-shrinking mortar or other appropriate material; 2) inlet and outlet pipes be joined to the manhole with a gasketed flexible watertight connection or another watertight connection arrangement that allows differential settlement of the pipe and manhole wall; and 3) watertight manhole covers be used wherever the manhole tops may be flooded by street runoff or high water. [RSWF 34.6]
CS	25	Manhole inspection and testing for water-tightness or damage prior to placing into service are specified. Air testing, if specified for concrete sewer manholes, conforms to the test procedures described in ASTM C-1244. [RSWF 34.7]
NA	26	Electrical equipment specified for use in manholes is consistent with Item 46 of this checklist. [RSWF 34.9]

### Stream Crossings

Initials (or "NA" or "NC")	Item Number	Requirement
NA	27	Sewers and force mains entering or crossing streams are designed to be constructed of ductile iron pipe with mechanical joints or so they will remain watertight and free from changes in alignment or grade or constructed of HDPE with fused joints for directional drilling. Appropriate materials which will not readily erode, cause siltation, damage pipe during placement, or corrode the pipe are specified to backfill the trench. [RSWF 36.21]
NA	28	Stream crossings are designed to incorporate valves or other flow regulating devices (which may include pump stations) on the shoreline or at such distances from the shoreline to prevent discharge in the event the line is damaged. [62-604.400(2)(j)5., F.A.C.]
NA	29	Sewers and force mains entering or crossing streams are designed at a sufficient depth below the natural bottom of the stream bed to protect the line. At a minimum, the project is designed with subaqueous lines to be buried at least three feet below the design or actual bottom, whichever is deeper, of a canal and other dredged waterway or the natural bottom of streams, rivers, estuaries, bays, and other natural water bodies; or if it is not practicable to design the project with less than three-foot minimum cover, alternative construction features (e.g. a concrete cap, sleeve, or some other properly engineered device to insure adequate protection of the line) are described in Part II.C. [62-604.400(2)(j)1., F.A.C., and RSWF 36.11]
NA	30	Specifications require permanent warning signs be placed on the banks of canals, streams, and rivers clearly identifying the nature and location (including depths below design or natural bottom) of subaqueous crossings and suitably fixed signs be placed at the shore, for subaqueous crossings of lakes, bays, and other large bodies of water, and in any area where anchoring is normally expected. [62-604.400(2)(j)2., F.A.C.]
NA	31	Provisions for testing the integrity of subaqueous lines are specified. [62-604.400(2)(j)4., F.A.C.]
NA	32	Supports are designed for all joints in pipes utilized for aerial crossings and to prevent overturning and settlement. Expansion jointing is specified between above ground and below ground sewers and force mains. The design considers the impact of floodwaters and debris. [RSWF 37]
NA	33	Aerial crossings are designed to maintain existing or required navigational capabilities within the waterway and to reserve riparian rights of adjacent property owners. [62-604.400(2)(j)3., F.A.C.]

### Pump Stations

Initials (or "NA" or "NC")	Item Number	Requirement
CS	34	In areas with high water tables, pump stations are designed to withstand flotation forces when empty. When siting the pump station, the design considers the potential for damage or interruption of operation because of flooding. Pump station structures and electrical and mechanical equipment are designed to be protected from physical damage by the 100-year flood. Pump stations are designed to remain fully operational and accessible during the 25-year flood unless lesser flood levels are appropriate based on local considerations, but not less than the 10-year flood. [62-604.400(2)(e), F.A.C.]
CS	35	Pump stations are designed to be readily accessible by maintenance vehicles during all weather conditions. [RSWF 41.2]
CS	36	Wet well and pump station piping is designed to avoid operational problems from the accumulation of grit. [RSWF 41.3]
NA	37	Dry wells, including their superstructure, are designed to be completely separated from the wet well. Common walls are designed to be gas tight. [RSWF 42.21]
CS	38	The design includes provisions to facilitate removing pumps, motors, and other mechanical and electrical equipment. [RSWF 42.22]
	39	The design includes provisions for: 1) suitable and safe means of access for persons wearing self-



Initials (or "NA" or "NC")	Item Number	Requirement
CS		contained breathing apparatus are provided to dry wells, and to wet wells; 2) stairway access to wet wells more than 4 feet deep containing either bar screens or mechanical equipment requiring inspection or maintenance; 3) for built-in-place pump stations, a stairway to the dry well with rest landings at vertical intervals not to exceed 12 feet; 4) for factory-built pump stations over 15 feet deep, a rigidly fixed landing at vertical intervals not to exceed 10 feet unless a manlift or elevator is provided; and 5) where a landing is used, a suitable and rigidly fixed barrier to prevent an individual from falling past the intermediate landing to a lower level. If a manlift or elevator is provided, emergency access is included in the design. [RSWF 42.23]
CS	40	Specified construction materials are appropriate under conditions of exposure to hydrogen sulfide and other corrosive gases, greases, oils, and other constituents frequently present in wastewater. [RSWF 42.25]
CS	41	Multiple pumps are specified, and each pump has an individual intake. Where only two units are specified, they are of the same size. Specified units have capacity such that, with any unit out of service, the remaining units will have capacity to handle the design peak hourly flow. [RSWF 42.31 and 42.36]
NA	42	Bar racks are specified for pumps handling wastewater from 30 inch or larger diameter sewers. Where a bar rack is specified, a mechanical hoist is also provided. The design includes provisions for appropriate protection from clogging for small pump stations. [RSWF 42.322]
CS	43	Pumps handling raw wastewater are designed to pass spheres of at least 3 inches in diameter. Pump suction and discharge openings are designed to be at least 4 inches in diameter. Note, this provision is not applicable to grinder pumps. [RSWF 42.33]
CS	44	The design requires pumps be placed such that under normal operating conditions they will operate under a positive suction head, unless pumps are suction-lift pumps. [RSWF 42.34]
CS	45	The design requires: 1) pump stations be protected from lightning and transient voltage surges; and 2) pump stations be equipped with lightning arrestors, surge capacitors, or other similar protection devices and phase protection. Note, small pump stations serving a single building are not required to provide surge protection devices if not necessary because the pump station is protected by the surge protection device of the single building. [62-604.400(2)(b), F.A.C.]
CS	46	The design requires 1) electrical systems and components (e.g., motors, lights, cables, conduits, switch boxes, control circuits, etc.) in raw wastewater wet wells, or in enclosed or partially enclosed spaces where hazardous concentrations of flammable gases or vapors may be present, comply with the National Electrical Code requirements; 2) electrical equipment located in wet wells be suitable for use under corrosive conditions; 3) each flexible cable be provided with a watertight seal and separate strain relief; 4) a fused disconnect switch located above ground be provided for the main power feed for all pump stations; 5) electrical equipment exposed to weather to meet the requirements of weatherproof equipment NEMA 3R or 4; 6) a 110 volt power receptacle to facilitate maintenance be provided inside the control panel for pump stations that have control panels outdoors; and 7) ground fault interruption protection be provided for all outdoor outlets. [RSWF 42.35]
NA	47	The design requires a sump pump equipped with dual check valves be provided in dry wells to remove leakage or drainage with discharge above the maximum high water level of the wet well. [RSWF 42.37]
CS	48	Pump/pump station design capacities are based on the peak hourly flow and are adequate to maintain a minimum velocity of 2 feet per second in the force main. [RSWF 42.38]
CS	49	The design includes provisions to automatically alternate the pumps in use. [RSWF 42.4]
CS	50	The design requires: 1) suitable shutoff valves be placed on the suction line of pumps/dry pit pumps; 2) suitable shutoff and check valves be placed on the discharge line of each pump (except on screw pumps); 3) a check valve be located between the shutoff valve and the pump; 4) check valves be suitable for the material being handled; 5) check valves be placed on the horizontal portion of discharge piping (except for ball checks, which may be placed in the vertical run); 6) all valves be capable of withstanding normal pressure and

Initials (or "NA" or "NC")	Item Number	Requirement
		water hammer; and 7) all shutoff and check valves be operable from the floor level and accessible for maintenance. [RSWF 42.5]
CS	51	The effective volume of wet wells is based on design average flows and a filling time not to exceed 30 minutes unless the facility is designed to provide flow equalization. The pump manufacturer's duty cycle recommendations were utilized in selecting the minimum cycle time. [RSWF 42.62]
CS	52	The design requires wet well floors have a minimum slope of 1 to 1 to the hopper bottom and the horizontal area of hopper bottoms be no greater than necessary for proper installation and function of the inlet. [RSWF 42.63]
CS	53	For covered wet wells, the design provides for air displacement to the atmosphere, such as an inverted "J" tube or other means. [RSWF 42.64]
CS	54	The design provides for adequate ventilation at all pump stations. Mechanical ventilation shall be provided where the dry well is below the ground surface. Permanently installed ventilation shall be provided if screens or mechanical equipment requiring maintenance or inspection are located in the wet well. Pump stations are designed with no interconnection between the wet well and dry well ventilation systems. [RSWF 42.71]
CS	55	The design requires all intermittently operated ventilation equipment to be interconnected with the respective pit lighting system and the manual lighting/ventilation switch to override the automatic controls. [RSWF 42.73]
NA	56	The design requires the fan wheels of ventilation systems be fabricated from non-sparking material and automatic heating and dehumidification equipment be provided in all dry wells. [RSWF 42.74]
NA	57	If wet well ventilation is continuous, design provides for at least 12 complete 100% fresh air changes per hour; if wet well ventilation is intermittent, design provides for at least 30 complete 100% fresh air changes per hour; and design requires air to be forced into wet wells by mechanical means rather than solely exhausted from the wet well. [RSWF 42.75]
NA	58	If dry well ventilation is continuous, design provides at least 12 complete 100% fresh air changes per hour; and dry well ventilation is intermittent, design provides for at least 30 complete 100% fresh air changes per hour, unless a system of two speed ventilation with an initial ventilation rate of 30 changes per hour for 10 minutes and automatic switch over to 6 changes per hour is used to conserve heat. [RSWF 42.76]
CS	59	Pump stations are designed and located on the site to minimize adverse effects from odors, noise, and lighting. [62-604.400(2)(c), F.A.C.]
CS	60	The design requires pump stations be enclosed with a fence or otherwise designed with appropriate features to discourage the entry of animals and unauthorized persons. Posting of an unobstructed sign made of durable weather resistant material at a location visible to the public with a telephone number for a point of contact in case of emergency is specified. [62-604.400(2)(d), F.A.C.]
CS	61	The design requires suitable devices for measuring wastewater flow at all pump stations. Indicating, totalizing, and recording flow measurement are specified for pump stations with a 350 gpm or greater design peak flow. [RSWF 42.8]
CS	62	The project is designed with no physical connections between any potable water supplies and pump stations. If a potable water supply is brought to a station, reduced-pressure principle backflow-prevention assemblies are specified. [RSWF 42.9 and 62-555.30(4), F.A.C.]

Additional Items to be Completed for Suction-Lift Pump Stations

Initials (or "NA" or "NC")	Item Number	Requirement
NA	63	The design requires all suction-lift pumps to be either self-priming or vacuum-priming and the combined total of dynamic suction-lift at the "pump off" elevation and required net positive suction head at design operating conditions not to exceed 22 feet. For self-priming

Initials (or "NA" or "NC")	Item Number	Requirement
		pumps, the design requires: 1) pumps be capable of rapid priming and repriming at the "lead pump on" elevation with self-priming and repriming accomplished automatically under design operating conditions; 2) suction piping not to exceed the size of the pump suction or 25 feet in total length; and 3) priming lift at the "lead pump on" elevation to include a safety factor of at least 4 feet from the maximum allowable priming lift for the specific equipment at design operating conditions. For vacuum-priming pump stations, the design requires dual vacuum pumps capable of automatically and completely removing air from the suction-lift pumps and the vacuum pumps be adequately protected from damage due to wastewater. [RSWF 43.1]
NA	64	The design requires: 1) suction-lift pump equipment compartments to be above grade or offset and to be effectively isolated from the wet well to prevent a hazardous and corrosive sewer atmosphere from entering the equipment compartment; 2) wet well access not to be through the equipment compartment and to be at least 24 inches in diameter; 3) gasketed replacement plates be provided to cover the opening to the wet well for pump units to be removed for service; and 4) no valving be located in the wet well. [RSWF 43.2]

Additional Items to be Completed for Submersible Pump Stations

Initials (or "NA" or "NC")	Item Number	Requirement
CS	65	Submersible pumps and motors are designed specifically for raw wastewater use, including totally submerged operation during a portion of each pump cycle and to meet the requirements of the National Electrical Code for such units. Provisions for detecting shaft seal failure or potential seal failure are included in the design. [RSWF 44.1]
CS	66	The design requires submersible pumps be readily removable and replaceable without dewatering the wet well or disconnecting any piping in the wet well. [RSWF 44.2]
CS	67	In submersible pump stations, electrical supply, control, and alarm circuits are designed to provide strain relief; to allow disconnection from outside the wet well; and to protect terminals and connectors from corrosion by location outside the wet well or through use of watertight seals. [RSWF 44.31]
CS	68	In submersible pump stations, the design requires the motor control center to be located outside the wet well, readily accessible, and protected by a conduit seal or other appropriate measures meeting the requirements of the National Electrical Code, to prevent the atmosphere of the wet well from gaining access to the control center. If a seal is specified, the motor can be removed and electrically disconnected without disturbing the seal. The design requires control equipment exposed to weather to meet the requirements of weatherproof equipment NEMA 3R or 4. [RSWF 44.32]
CS	69	In submersible pump stations, the design requires: 1) pump motor power cords be flexible and serviceable under conditions of extra hard usage and to meet the requirements of the National Electrical Code standards for flexible cords in wastewater pump stations; 2) ground fault interruption protection be used to de-energize the circuit in the event of any failure in the electrical integrity of the cable; and 3) power cord terminal fittings be corrosion-resistant and constructed in a manner to prevent the entry of moisture into the cable, provided with strain relief appurtenances, and designed to facilitate field connecting. [RSWF 44.33]
CS	70	In submersible pump stations, the design requires all shut-off and check valves be located in a separate valve pit. Provisions to remove or drain accumulated water from the valve pit are included in the design. [RSWF 44.4]



### Emergency Operations for Pump Stations

Initials (or "NA" or "NC")	Item Number	Requirement
CS	71	Pump stations are designed with an alarm system which activates in cases of power failure, sump pump failure, pump failure, unauthorized entry, or any cause of pump station malfunction. Pump station alarms are designed to be telemetered to a facility that is manned 24 hours a day. If such a facility is not available and a 24-hour holding capacity is not provided, the alarm is designed to be telemetered to utility offices during normal working hours and to the home of the responsible person(s) in charge of the lift station during off-duty hours. Note, if an audio-visual alarm system with a self-contained power supply is provided in lieu of a telemetered system, documentation is provided in Part II.(5)B. showing an equivalent level of reliability and public health protection. [RSWF 46]
CS	72	The design requires emergency pumping capability be provided for all pump stations. For pump stations that receive flow from one or more pump stations through a force main or pump stations discharging through pipes 12 inches or larger, the design requires uninterrupted pumping capability be provided, including an in-place emergency generator. Where portable pumping and/or generating equipment or manual transfer is used, the design includes sufficient storage capacity with an alarm system to allow time for detection of pump station failure and transportation and connection of emergency equipment. [62-604.400(2)(a)1. and 2., F.A.C., and RSWF 47.423 and 47.433]
CS	73	The design requires: 1) emergency standby systems to have sufficient capacity to start up and maintain the total rated running capacity of the station, including lighting, ventilation, and other auxiliary equipment necessary for safety and proper operation; 2) special sequencing controls be provided to start pump motors unless the generating equipment has capacity to start all pumps simultaneously with auxiliary equipment operating; 3) a riser from the force main with rapid connection capabilities and appropriate valving be provided for all pump stations to hook up portable pumps; and 4) all pump station reliability design features be compatible with the available temporary service power generating and pumping equipment of the authority responsible for operation and maintenance of the collection/transmission system. [62-604.400(2)(a)3., F.A.C., and RSWF 47.431]
CS	74	The design provides for emergency equipment to be protected from operation conditions that would result in damage to the equipment and from damage at the restoration of regular electrical power. [RSWF 47.411, 47.417, and 47.432]
NA	75	Where independent substations are used for emergency power, each separate substation and its associated transmission lines is designed to be capable of starting and operating the pump station at its rated capacity. [RSWF 47.44]

### Force Mains

Initials (or "NA" or "NC")	Item Number	Requirement
CS	76	Force mains are designed to maintain, at design pumping rates, a cleansing velocity of at least 2 feet per second. The minimum force main diameter specified for raw wastewater is not less than 4 inches. (Not applicable to low pressure sewer systems) [RSWF 49.1]
CS	77	The design requires: 1) branches of intersecting force mains be provided with appropriate valves such that one branch may be shut down for maintenance and repair without interrupting the flow of other branches; and 2) stub-outs on force mains, placed in anticipation of future connections, be equipped with a valve to allow such connection without interruption of service. [62-604.400(2)(f), F.A.C.]
CS	78	The design requires air relief valves be placed at high points in the force main to prevent air locking. [RSWF 49.2]
CS	79	Specified force main pipe and joints are equal to water main strength materials suitable for design conditions. The force main, reaction blocking, and station piping are designed to withstand water hammer pressures and stresses associated with the cycling of wastewater

Initials (or "NA" or "NC")	Item Number	Requirement
		pump stations. [RSWF 49.4]
CS	80	When the Hazen and Williams formula is used to calculate friction losses through force mains, the value for "C" is 100 for unlined iron or steel pipe for design. For other smooth pipe materials, such as PVC, polyethylene, lined ductile iron, the value for C does not exceed 120 (130 for PVC and HDPE) for design. (Not applicable to low pressure sewer systems) [RSWF 49.61]
CS	81	Where force mains are constructed of material, which might cause the force main to be confused with potable water mains, specifications require the force main to be clearly identified. [RSWF 49.7]
CS	82	Leakage tests for force mains are specified including testing methods and leakage limits. [RSWF 49.8]

Note, if this project is an alternative collection system (i.e. a low pressure sewer system or a vacuum sewer system), complete the checklist items on Attachment I for low pressure sewer systems or Attachment II for vacuum sewer systems. Include the attachment with the submittal. For any items marked "NA" or "NC," provide an explanation in section 5(B).



- B. Explanation for Requirements or Standards Marked "NA" or "NC" in II(5)A above, which includes Attachments I and II (attach additional sheets if necessary):

11-12: Velocities not greater than 10 fps and no sewers with greater than 20% slope

19: No inverted siphons

27-33: No Stream Crossings

37,42,47,56,58: No dry wells

63-64: No suction lift pumps

75: No independent pump stations

14: NO JOINING OF DISSIMILAR MATERIAL FOR GRAVITY SEWER

57: NO DESIGNED VENTILATION (INVERTED J TUBE SIPHON VENTILATION)

**PART III - Certifications**

**(1) Collection/Transmission System Permittee**

I, the undersigned owner or authorized representative\* of The St. Joe Company  
am fully aware that the statements made in this application for a construction permit are true, correct and complete to the best of my knowledge and belief. I agree to retain the design engineer or another professional engineer registered in Florida, to conduct on-site observation of construction, to prepare a certification of completion of construction, and to review record drawings for adequacy. Further, I agree to provide an appropriate operation and maintenance manual for the facilities pursuant to Rule 62-604.500(4), F.A.C., and to retain a professional engineer registered in Florida to examine (or to prepare if desired) the manual. I am fully aware that Department approval must be obtained before this project is placed into service for any purpose other than testing for leaks and testing equipment operation.

Signed [Signature]  
Name Jason Scarbrough

Date 9.15.22  
Title Project Manager

\*Attach a letter of authorization.

**(2) Owner of Collection/Transmission System**

I, the undersigned owner or authorized representative\* of City of Port St. Joe certify that we will be the Owner of this project after it is placed into service. I agree that we will operate and maintain this project\*\* in a manner that will comply with applicable Department rules. Also, I agree that we will promptly notify the Department if we sell or legally transfer ownership of this project.

Signed \_\_\_\_\_  
Name Jim Anderson

Date \_\_\_\_\_  
Title City Manager

Company Name City of Port St. Joe

Address P.O. Box 278

City Port St. Joe

State FL

Zip 32457

Telephone 850-229-8261

Cell \_\_\_\_\_

Fax 850-229-8973

Email janderson@psj.fl.gov

\* Attach a letter of authorization

\*\*Description of the owner's portion if split \_\_\_\_\_

Second Owner of Collection/Transmission System (if system is divided with different owners)

I, the undersigned owner or authorized representative\* of \_\_\_\_\_ certify that we will be the Owner of this project after it is placed into service. I agree that we will operate and maintain this project in a manner that will comply with applicable Department rules. Also, I agree that we will promptly notify the Department if we sell or legally transfer ownership of this project.

Signed \_\_\_\_\_  
Name Kevin Pettis

Date \_\_\_\_\_  
Title Plant Manager

Company Name City of Port St. Joe Utility Services

Address P.O. Box 278

City Port St. Joe

State FL

Zip 32457

Telephone 850-229-8247

Cell \_\_\_\_\_

Fax 850-229-8973

Email kpettis@psj.fl.gov

\* Attach a letter of authorization

\*\*Description of the second owner portion if split \_\_\_\_\_

**(3) Wastewater Facility Serving Collection/Transmission System\*\***

If this is a Notice of Intent to use a general permit, check here:

☒ The undersigned owner or authorized representative\* of the City of Port St. Joe Wastewater Facility wastewater facility

hereby certifies that the above referenced facility has the capacity to receive the wastewater generated by the proposed collection system; is in compliance with the capacity analysis report requirements of Rule 62-600.405, F.A.C.; is not under a Department order associated with effluent violations or the ability to treat wastewater adequately; and will provide the necessary treatment and disposal as required by Chapter 403, F.S., and applicable Department rules.

If this is an application for an individual permit, check one:

☐ The undersigned owner or authorized representative\* of the \_\_\_\_\_ wastewater facility hereby certifies that the above referenced facility has and will have adequate reserve capacity to accept the flow from this project and will provide the necessary treatment and disposal as required by Chapter 403, F.S., and applicable Department rules.

☐ The undersigned owner or authorized representative\* of the \_\_\_\_\_ wastewater facility hereby certifies that the above referenced facility currently does not have, but will have prior to placing the proposed project into operation, adequate reserve capacity to accept the flow from this project and will provide the necessary treatment and disposal as required by Chapter 403, F.S., and applicable Department rules.

Name of Treatment Plant Serving Project City of Port St. Joe

County Gulf City Port St. Joe

DEP Facility ID: FL 020206

Maximum monthly average daily flow over the last 12 month period \_\_\_\_\_ MGD Month(s) used \_\_\_\_\_

Maximum three-month average daily flow over the last 12 month period \_\_\_\_\_ MGD Month(s) used \_\_\_\_\_

Current permitted capacity \_\_\_\_\_ MGD ☐ AADF ☐ MADF ☐ TMADF

Current outstanding flow commitments (including this project) against treatment plant capacity \_\_\_\_\_ MGD

Signed \_\_\_\_\_ Date \_\_\_\_\_

Name Kevin Pettis Title Plant Manager

Company Name City of Port St. Joe Utility Services

Address P.O. Box 278

City Port St. Joe State FL Zip 32457

Telephone 850.229.8247 Cell \_\_\_\_\_ Fax \_\_\_\_\_

Email kpettis@psj.fl.gov


\* Attach a letter of authorization

\*\* If there is an intermediate satellite collection system between the project and the final receiving facility collection system, a letter shall be attached certifying that the intermediate downstream satellite collection system has adequate reserve capacity to accept the flow from this project.

(4) Professional Engineer Registered in Florida

I, the undersigned professional engineer registered in Florida, certify that I am in responsible charge of the preparation and production of engineering documents for this project; that plans and specifications for this project have been completed; that I have expertise in the design of wastewater collection/transmission systems; and that, to the best of my knowledge and belief, the engineering design for this project complies with the requirements of Chapter 62-604, F.A.C.

(Affix Seal)

Signed   
Date 10/25/22  
Name Christopher Shortt, P.E. Florida Registration No. 78424  
Company Name Dewberry Engineers Inc.  
Address 203 Aberdeen Parkway  
City Panama City State FL Zip 32405  
Telephone 850-571-1199 Cell \_\_\_\_\_ Fax \_\_\_\_\_  
Email cshortt@dewberry.com  
Portion of the project for which responsible: 100%

Second Engineer (if applicable)

(Affix Seal)

Signed \_\_\_\_\_  
Date \_\_\_\_\_  
Name \_\_\_\_\_ Florida Registration No. \_\_\_\_\_  
Company Name \_\_\_\_\_  
Address \_\_\_\_\_  
City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_  
Telephone \_\_\_\_\_ Cell \_\_\_\_\_ Fax \_\_\_\_\_  
Email \_\_\_\_\_  
Portion of the Project for Which Responsible: \_\_\_\_\_

Third Engineer (if applicable)

(Affix Seal)

Signed \_\_\_\_\_

Date \_\_\_\_\_

Name \_\_\_\_\_ Florida Registration No. \_\_\_\_\_

Company Name \_\_\_\_\_

Address \_\_\_\_\_

City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Telephone \_\_\_\_\_ Cell \_\_\_\_\_ Fax \_\_\_\_\_

Email \_\_\_\_\_

Portion of the Project for Which Responsible: \_\_\_\_\_

Fourth Engineer (if applicable)

(Affix Seal)

Signed \_\_\_\_\_

Date \_\_\_\_\_

Name \_\_\_\_\_ Florida Registration No. \_\_\_\_\_

Company Name \_\_\_\_\_

Address \_\_\_\_\_

City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Telephone \_\_\_\_\_ Cell \_\_\_\_\_ Fax \_\_\_\_\_

Email \_\_\_\_\_

Portion of the Project for Which Responsible: \_\_\_\_\_



## NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

**INSTRUCTIONS:** This notice shall be completed and submitted by persons proposing to construct projects permitted under the "General Permit for Construction of Water Main Extensions for Public Water Systems" in Rule 62-555.405, F.A.C. AT LEAST 30 DAYS BEFORE BEGINNING CONSTRUCTION OF A WATER MAIN EXTENSION PROJECT, complete and submit one copy of this notice to the appropriate Department of Environmental Protection (DEP) District Office or Approved County Health Department (ACHD) along with payment of the proper permit processing fee. (When completed, Part II of this notice serves as the preliminary design report for a water main extension project, and thus, it is unnecessary to submit a separate preliminary design report or drawings, specifications, and design data with this notice.) All information provided in this notice shall be typed or printed in ink. The DEP permit processing fee for projects requiring the services of a professional engineer during design is \$650, and the DEP permit processing fee for projects not requiring the services of a professional engineer during design is \$500.\* Some ACHDs charge a county permit processing fee in addition to the DEP permit processing fee. Checks for permit processing fees shall be made payable to the Department of Environmental Protection or the appropriate ACHD. NOTE THAT A SEPARATE NOTIFICATION AND A SEPARATE PERMIT PROCESSING FEE ARE REQUIRED FOR EACH NON-CONTIGUOUS PROJECT.†

\* Except as noted in paragraphs 62-555.520(3)(a) and (b), F.A.C., projects shall be designed under the responsible charge of one or more professional engineers licensed in Florida.

† Non-contiguous projects are projects that are neither interconnected nor located nearby one another (i.e., on the same site, on adjacent streets, or in the same neighborhood).

### I. General Project Information

A. Name of Project: **Windmark Beach North Townhomes**

B. Description of Project and Its Purpose:

The project is the development of 78 townhome residential lots with supporting roadways, a stormwater management facility, and utilities. A water distribution system will be constructed on site to provide potable flows and fire protection. System components include, but are not limited to: 1,479 LF of 6" WM, 79 water meters, 4 fire hydrant assemblies, and all necessary valves and appurtenances.

C. Location of Project

1. County Where Project Located: Gulf

2. Description of Project Location:

The project is located north of the Windmark Beach developments in Port St. Joe, Florida. The project is located on the north side US HWY 98.

D. Estimate of Cost to Construct Project: \$120,000

E. Estimate of Dates for Starting and Completing Construction of Project:

May 2023-December 2023

F. Permittee

PWS/Company Name: <u>The St. Joe Company</u>		PWS Identification No.: *		
PWS Type: *	<input checked="" type="checkbox"/> Community	<input type="checkbox"/> Non-Transient Non-Community	<input type="checkbox"/> Transient Non-Community	<input type="checkbox"/> Consecutive
Contact Person: <u>Jason Scarbrough</u>		Contact Person's Title: <u>Project Manager</u>		
Contact Person's Mailing Address: <u>130 Richard Jackson Blvd, Suite 200</u>				
City: <u>Panama City Beach</u>		State: <u>FL</u>	Zip Code: <u>32407</u>	
Contact Person's Telephone Number: <u>850-231-6530</u>		Contact Person's Fax Number:		
Contact Person's E-Mail Address: <u>jason.scarbrough@joe.com</u>				

\* This information is required only if the permittee is a public water system (PWS).

G. Public Water System (PWS) Supplying Water to Project

PWS Name: <u>City of Port St. Joe</u>		PWS Identification No.:		
PWS Type:	<input checked="" type="checkbox"/> Community	<input type="checkbox"/> Non-Transient Non-Community	<input type="checkbox"/> Transient Non-Community	<input type="checkbox"/> Consecutive
PWS Owner: <u>City of Port St. Joe</u>		Contact Person's Title: <u>City Manager</u>		
Contact Person: <u>Jim Anderson</u>		Contact Person's Title: <u>City Manager</u>		
Contact Person's Mailing Address: <u>P.O. Box 278</u>				
City: <u>Port St. Joe</u>		State: <u>FL</u>	Zip Code: <u>32467</u>	
Contact Person's Telephone Number: <u>850-226-6261</u>		Contact Person's Fax Number:		
Contact Person's E-Mail Address: <u>janderson@psj.fl.gov</u>				



# NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

Project Name: Windward Beach North Townhomes Permittee: The St. Joe Company

## H. Public Water System (PWS) that Will Own Project After It Is Placed into Permanent Operation

PWS Name: City of Port St. Joe		PWS Identification No.: * 1230545	
PWS Type: *	<input type="checkbox"/> Community <input type="checkbox"/> Non-Transient Non-Community <input type="checkbox"/> Transient Non-Community <input type="checkbox"/> Consecutive		
PWS Owner: City of Port St. Joe			
Contact Person: Lany McClamma		Contact Person's Title: Plant Manager	
Contact Person's Mailing Address: P.O. Box 278			
City: Port St. Joe		State: FL	Zip Code: 32457
Contact Person's Telephone Number: 850-229-8247		Contact Person's Fax Number: 850-229-7522	
Contact Person's E-Mail Address:			

\* This information is required only if the owner/operator is an existing PWS.

## I. Professional Engineer(s) or Other Person(s) in Responsible Charge of Designing Project\*

Company Name: Dewberry Engineers Inc	
Designer(s): Christopher Shortt, PE	Title(s) of Designer(s): Project Manager
Qualifications of Designer(s):	
<input checked="" type="checkbox"/> Professional Engineer(s) Licensed in Florida – License Number(s): 78424	
<input type="checkbox"/> Public Officer(s) Employed by State, County, Municipal, or Other Governmental Unit of State†	
<input type="checkbox"/> Plumbing Contractor(s) Licensed in Florida – License Number(s): ^	
Mailing Address of Designer(s): 203 Aberdeen Pkwy.	
City: Panama City	State: FL Zip Code: 32405
Telephone Number of Designer(s): 850-571-1199	Fax Number of Designer(s):
E-Mail Address(es) of Designer(s): cshortt@dewberry.com	

\* Except as noted in paragraphs 62-555.520(3)(a) and (b), F.A.C., projects shall be designed under the responsible charge of one or more professional engineers licensed in Florida.

† Attach a detailed construction cost estimate showing that the cost to construct this project is \$10,000 or less.

^ Attach documentation showing that this project will be installed by the plumbing contractor(s) designing this project, documentation showing that this project involves a public water system serving a single property and fewer than 250 fixture units, and a detailed construction cost estimate showing that the cost to construct this project is \$50,000 or less.

## II. Preliminary Design Report for Project\*

### A. Service Area, Water Use, and Service Pressure Information

1. Design Type and Number of Service Connections, and Average Daily Water Demands and Maximum-Day Water Demands, in the Entire Area to Be Served by the Water Mains Being Constructed Under this Project:

A = Type of Service Connection	B = Number of Service Connections	C = Average Daily Water Demand Per Service Connection, gpd	D = Total Average Daily Water Demand <sup>a</sup> , gpd (Columns BxC for Residential Service Connections)	E = Total Maximum-Day Water Demand <sup>b</sup> , gpd
Single-Family Home	78	250	19500	28250
Mobile Home			0	
Apartment			0	
Commercial, Institutional, or Industrial Facility <sup>a</sup>				
Total	78		19500	28250

- a. Description of Commercial, Institutional, or Industrial Facilities and Explanation of Method(s) Used to Estimate Average Daily Water Demand for These Facilities:

N/A

- b. Explanation of Peaking Factor(s) or Method(s) Used to Estimate Maximum-Day Water Demand:

The maximum day water demand has been estimated utilizing the average daily water demand multiplied by 1.5

## NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSS

Project Name: Windmark Beach North Townhome

Permittee: The St. Joe Company

2. Explanation of Peaking Factor(s) or Method(s) Used to Estimate Design Peak-Hour Water Demand and, for Small Water Systems that Use Hydropneumatic Tanks or that Are Not Designed to Provide Fire Protection, Peak Instantaneous Water Demand:  
A peaking factors 4.0 has been used to determine peak hour demand. This results in a peak hour demand of 3250 GPH.

3. Design Fire-Flow Rate and Duration:  
1000 gpm for 2 hours

4. Design Service Pressure Range:  
40-80 psi

### B. Project Site Information

1. ATTACH A SITE PLAN OR SKETCH SHOWING THE SIZE AND APPROXIMATE LOCATION OF NEW OR ALTERED WATER MAINS, SHOWING THE APPROXIMATE LOCATION OF HYDRANTS, VALVES, METERS, AND BLOW-OFFS IN SAID MAINS, AND SHOWING HOW SAID MAINS CONNECT TO THE PUBLIC WATER SYSTEM SUPPLYING WATER FOR THE PROJECT.
2. Description of Any Areas Where New or Altered Water Mains Will Cross Above or Under Surface Water or Be Located in Soil that Is Known to Be Aggressive:  
No stream crossings are proposed, soils in this area are not known to be aggressive

### C. Information About Compliance with Design and Construction Requirements

1. If this project is being designed to comply with the following requirements, initial in ink before the requirements. If any of the following requirements do not apply to this project or if this project includes exceptions to any of the following requirements as allowed by rule, mark "X" before the requirements and complete Part II.C.2 below. *RSWW* = *Recommended Standards for Water Works* as incorporated into Rule 62-555.330, F.A.C.

- CS a. This project is being designed to keep existing water mains and service lines in operation during construction or to minimize interruption of water service during construction. [RSWW 1.3.a; exceptions allowed under FAC 62-555.330]
- CS b. All pipe, pipe fittings, pipe joint packing and jointing materials, valves, fire hydrants, and meters installed under this project will conform to applicable American Water Works Association (AWWA) standards. [FAC 62-555.320(21)(b), RSWW 8.0, and AWWA standards as incorporated into FAC 62-555.330; exceptions allowed under FAC 62-555.320(21)(c)]
- CS c. All public water system components, excluding fire hydrants, that will be installed under this project and that will come into contact with drinking water will conform to NSF International Standard 61 as adopted in Rule 62-555.335, F.A.C., or other applicable standards, regulations, or requirements referenced in paragraph 62-555.320(3)(b), F.A.C. [FAC 62-555.320(3)(b); exceptions allowed under FAC 62-555.320(3)(d)]
- CS d. All pipe and pipe fittings installed under this project will contain no more than 8.0% lead, and any solder or flux used in this project will contain no more than 0.2% lead. [FAC 62-555.322]
- CS e. All pipe and pipe fittings installed under this project will be color coded or marked in accordance with subparagraph 62-555.320(21)(b)3, F.A.C., using blue as a predominant color. (Underground plastic pipe will be solid-wall blue pipe, will have a co-extruded blue external skin, or will be white or black pipe with blue stripes incorporated into, or applied to, the pipe wall; and underground metal or concrete pipe will have blue stripes applied to the pipe wall. Pipe striped during manufacturing of the pipe will have continuous stripes that run parallel to the axis of the pipe, that are located at no greater than 90-degree intervals around the pipe, and that will remain intact during and after installation of the pipe. If tape or paint is used to stripe pipe during installation of the pipe, the tape or paint will be applied in a continuous line that runs parallel to the axis of the pipe and that is located along the top of the pipe; for pipe with an internal diameter of 24 inches or greater, tape or paint will be applied in continuous lines along each side of the pipe as well as along the top of the pipe. Aboveground pipe will be painted blue or will be color coded or marked like underground pipe.) [FAC 62-555.320(21)(b)3]
- CS f. All new or altered water mains included in this project are sized after a hydraulic analysis based on flow demands and pressure requirements. ATTACH A HYDRAULIC ANALYSIS JUSTIFYING THE SIZE OF ANY NEW OR ALTERED WATER MAINS WITH AN INSIDE DIAMETER OF LESS THAN THREE INCHES. [FAC 62-555.320(21)(b) and RSWW 8.1]

# **NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSSs**

**Project Name:** Windmark Beach North Trenches

**Permittee:** The St. Joe Company

- CS g. The inside diameter of new or altered water mains that are included in this project and that are being designed to provide fire protection and serve fire hydrants will be at least six inches. [FAC 62-555.320(21)(b) and *RSWW* 8.1.2]
- CS h. New or altered water mains that are included in this project and that are not being designed to carry fire flows do not have fire hydrants connected to them. [FAC 62-555.320(21)(b) and *RSWW* 8.1.5]
- CS i. This project is being designed to minimize dead-end water mains by making appropriate tie-ins where practical. [FAC 62-555.320(21)(b) and *RSWW* 8.1.6.a]
- CS j. New or altered dead-end water mains included in this project will be provided with a fire or flushing hydrant or blow-off for flushing purposes. [FAC 62-555.320(21)(b) and *RSWW* 8.1.6.b]
- CS k. Sufficient valves will be provided on new or altered water mains included in this project so that inconvenience and sanitary hazards will be minimized during repairs. [FAC 62-555.320(21)(b) and *RSWW* 8.2]
- CS l. New or altered fire hydrant leads included in this project will have an inside diameter of at least six inches and will include an auxiliary valve. [FAC 62-555.320(21)(b) and *RSWW* 8.3.3]
- CS m. All fire hydrants that will be installed under this project and that will have unplugged, underground drains will be located at least three feet from any existing or proposed storm sewer, stormwater force main, pipeline conveying reclaimed water regulated under Part III of Chapter 62-610, F.A.C., or vacuum-type sanitary sewer; at least six feet from any existing or proposed gravity- or pressure-type sanitary sewer, wastewater force main, or pipeline conveying reclaimed water not regulated under Part III of Chapter 62-10, F.A.C.; and at least ten feet from any existing or proposed "on-site sewage treatment and disposal system." [FAC 62-555.314(4)]
- CS n. At high points where air can accumulate in new or altered water mains included in this project, provisions will be made to remove the air by means of air relief valves, and automatic air relief valves will not be used in situations where flooding of the valve manhole or chamber may occur. [FAC 62-555.320(21)(b) and *RSWW* 8.4.1]
- CS o. The open end of the air relief pipe from all automatic air relief valves installed under this project will be extended to at least one foot above grade and will be provided with a screened, downward-facing elbow. [FAC 62-555.320(21)(b) and *RSWW* 8.4.2]
- CS p. New or altered chambers, pits, or manholes that contain valves, blow-offs, meters, or other such water distribution system appurtenances and that are included in this project will not be connected directly to any sanitary or storm sewer, and blow-offs or air relief valves installed under this project will not be connected directly to any sanitary or storm sewer. [FAC 62-555.320(21)(b) and *RSWW* 8.4.3]
- CS q. New or altered water mains included in this project will be installed in accordance with applicable AWWA standards or in accordance with manufacturers' recommended procedures. [FAC 62-555.320(21)(b), *RSWW* 8.5.1, and AWWA standards as incorporated into FAC 62-555.330]
- CS r. A continuous and uniform bedding will be provided in trenches for underground pipe installed under this project; backfill material will be tamped in layers around underground pipe installed under this project and to a sufficient height above the pipe to adequately support and protect the pipe; and unsuitably sized stones (as described in applicable AWWA standards or manufacturers' recommended installation procedures) found in trenches will be removed for a depth of at least six inches below the bottom of underground pipe installed under this project. [FAC 62-555.320(21)(b), *RSWW* 8.5.2]
- CS s. All water main tees, bends, plugs, and hydrants installed under this project will be provided with thrust blocks or restrained joints to prevent movement. [FAC 62-555.320(21)(b) and *RSWW* 8.5.4]
- CS t. New or altered water mains that are included in this project and that will be constructed of asbestos-cement or polyvinyl chloride pipe will be pressure and leakage tested in accordance with AWWA Standard C603 or C605, respectively, as incorporated into Rule 62-555.330, F.A.C., and all other new or altered water mains included in this project will be pressure and leakage tested in accordance with AWWA Standard C600 as incorporated into Rule 62-555.330. [FAC 62-555.320(21)(b)1 and AWWA standards as incorporated into FAC 62-555.330]
- CS u. New or altered water mains, including fire hydrant leads and including service lines that will be under the control of a public water system and that have an inside diameter of three inches or greater, will be disinfected and bacteriologically evaluated in accordance with Rule 62-555.340, F.A.C. [FAC 62-555.320(21)(b)2 and FAC 62-555.340]
- CS v. New or altered water mains that are included in this project and that will be installed in areas where there are known aggressive soil conditions will be protected through use of corrosion-resistant water main materials, through encasement of the water mains in polyethylene, or through provision of cathodic protection. [FAC 62-555.320(21)(b) and *RSWW* 8.5.7 d]



# NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

Project Name: Windmark Beach North, Townhomes

Permittee: The St. Joe Company

CS

w. New or relocated, underground water mains included in this project will be laid to provide a horizontal distance of at least three feet between the outside of the water main and the outside of any existing or proposed vacuum-type sanitary sewer, storm sewer, stormwater force main, or pipeline conveying reclaimed water regulated under Part III of Chapter 62-610, F.A.C.; a horizontal distance of at least six feet between the outside of the water main and the outside of any existing or proposed gravity-type sanitary sewer (or a horizontal distance of at least three feet between the outside of the water main and the outside of any existing or proposed gravity-type sanitary sewer if the bottom of the water main will be laid at least six inches above the top of the sewer); a horizontal distance of at least six feet between the outside of the water main and the outside of any existing or proposed pressure-type sanitary sewer, wastewater force main, or pipeline conveying reclaimed water not regulated under Part III of Chapter 62-610, F.A.C.; and a horizontal distance of at least ten feet between the outside of the water main and all parts of any existing or proposed "on-site sewage treatment and disposal system." [FAC 62-555.314(1); exceptions allowed under FAC 62-555.314(5)]

CS

x. New or relocated, underground water mains that are included in this project and that will cross any existing or proposed gravity- or vacuum-type sanitary sewer or storm sewer will be laid so the outside of the water main is at least six inches above the other pipeline or at least 12 inches below the other pipeline; and new or relocated, underground water mains that are included in this project and that will cross any existing or proposed pressure-type sanitary sewer, wastewater or stormwater force main, or pipeline conveying reclaimed water will be laid so the outside of the water main is at least 12 inches above or below the other pipeline. [FAC 62-555.314(2); exceptions allowed under FAC 62-555.314(5)]

CS

y. At the utility crossings described in Part II.C.1.w above, one full length of water main pipe will be centered above or below the other pipeline so the water main joints will be as far as possible from the other pipeline or the pipes will be arranged so that all water main joints are at least three feet from all joints in vacuum-type sanitary sewers, storm sewers, stormwater force mains, or pipelines conveying reclaimed water regulated under Part III of Chapter 62-610, F.A.C., and at least six feet from all joints in gravity- or pressure-type sanitary sewers, wastewater force mains, or pipelines conveying reclaimed water not regulated under Part III of Chapter 62-610, F.A.C. [FAC 62-555.314(2); exceptions allowed under FAC 62-555.314(5)]

\*

z. New or altered water mains that are included in this project and that will cross above surface water will be adequately supported and anchored, protected from damage and freezing, and accessible for repair or replacement. [FAC 62-555.320(21)(b) and RSWW 8.7.1]

\*

aa. New or altered water mains that are included in this project and that will cross under surface water will have a minimum cover of two feet. [FAC 62-555.320(21)(b) and RSWW 8.7.2]

\*

bb. New or altered water mains that are included in this project and that will cross under surface water courses greater than 15 feet in width will have flexible or restrained, watertight pipe joints and will include valves at both ends of the water crossing so the underwater main can be isolated for testing and repair; the aforementioned isolation valves will be easily accessible and will not be subject to flooding; the isolation valve closest to the water supply source will be in a manhole; and permanent taps will be provided on each side of the isolation valve within the manhole to allow for insertion of a small meter to determine leakage from the underwater main and to allow for sampling of water from the underwater main. [FAC 62-555.320(21)(b) and RSWW 8.7.2]

CS

cc. This project is being designed to include proper backflow protection at those new or altered service connections where backflow protection is required or recommended under Rule 62-555.360, F.A.C., or in *Recommended Practice for Backflow Prevention and Cross-Connection Control*, AWWA Manual M14, as incorporated into Rule 62-555.330, F.A.C.; or the public water system that will own this project after it is placed into operation has a cross-connection control program requiring water customers to install proper backflow protection at those service connections where backflow protection is required or recommended under Rule 62-555.360, F.A.C., or in AWWA Manual M14. [FAC 62-555.360 and AWWA Manual M14 as incorporated into FAC 62-555.330]

CS

dd. Neither steam condensate, cooling water from engine jackets, nor water used in conjunction with heat exchangers will be returned to the new or altered water mains included in this project. [FAC 62-555.320(21)(b) and RSWW 8.8.2]

# NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

Project Name: Windsor Beach North Townhomes

Permittee: The St. Joe Company

2. Explanation for Requirements Marked "X" in Part II.C.1 Above, Including Justification, Documentation, Assurances, and/or Alternatives as Required by Rule for Exceptions to Requirements in Part II.C.1:

- v. No Known aggressive soils
- z.-bb. No surface water crossings

I completed Part II of this notice, and the information provided in Part II and on the attachment(s) to Part II is true and accurate to the best of my knowledge and belief.

Signature, Seal, and Date of Professional Engineer (PE) or  
Signature and Date of Other Person in Responsible Charge of  
Designing Project:\*



Printed/Typed Name: Christopher Shortt, PE

License Number of PE or License Number or Title of Other  
Person in Responsible Charge of Designing Project:\*

78424

Portion of Preliminary Design Report for Which Responsible:  
All

Signature, Seal, and Date of Professional Engineer (PE) or  
Signature and Date of Other Person in Responsible Charge of  
Designing Project:\*

Printed/Typed Name:

License Number of PE or License Number or Title of Other  
Person in Responsible Charge of Designing Project:\*

Portion of Preliminary Design Report for Which Responsible:

\* Except as noted in paragraphs 62-555.520(3)(a) and (b), F.A.C., projects shall be designed under the responsible charge of one or more PEs licensed in Florida. If this project is being designed under the responsible charge of one or more PEs licensed in Florida, Part II of this notice shall be completed, signed, sealed, and dated by the PE(s) in responsible charge. If this project is not being designed under the responsible charge of one or more PEs licensed in Florida, Part II shall be completed, signed, and dated by the person(s) in responsible charge of designing this project.

## NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

Project Name: Windmark North Townhomes

Permittee: The St. Joe Company

### III. Certifications


#### A. Certification by Permittee

I am duly authorized to sign this notice on behalf of the permittee identified in Part I.F of this notice. I certify that, to the best of my knowledge and belief, this project complies with Chapter 62-555, F.A.C. I also certify that construction of this project has not begun yet and that, to the best of my knowledge and belief, this project does not include any of the following construction work:

- construction of water mains conveying raw or partially treated drinking water;
- construction of drinking water treatment, pumping, or storage facilities or conflict manholes;
- construction of water mains in areas contaminated by low-molecular-weight petroleum products or organic solvents;
- construction of an interconnection between previously separate public water systems or construction of water mains that create a "new system" as described under subsection 62-555.525(1), F.A.C.; or
- construction of water mains that will remain dry following completion of construction.

(A specific construction permit is required for each project involving any of the above listed construction work.)

I understand that, if this project is designed under the responsible charge of one or more professional engineers (PEs) licensed in Florida, the permittee must retain a Florida-licensed PE to take responsible charge of inspecting construction of this project for the purpose of determining in general if the construction proceeds in compliance with the Department of Environmental Protection construction permit, including the approved preliminary design report, for this project. I understand that the permittee must have complete record drawings prepared for this project. I also understand that the permittee must submit a certification of construction completion to the Department and obtain written approval, or clearance, from the Department before the permittee places this project into operation for any purpose other than disinfection or testing for leaks.

 9.15.22  
Signature and Date

Jason Scarbrough

Printed or Typed Name

Project Manager

Title

#### B. Certification by PWS Supplying Water to Project

I am duly authorized to sign this notice on behalf of the PWS identified in Part I.G of this notice. I certify that said PWS will supply the water necessary to meet the design water demands for this project. As indicated below, the water treatment plant(s) to which this project will be connected has(have) the capacity necessary to meet the design water demands for this project, and I certify that all other PWS components affected by this project also have the capacity necessary to meet the design water demands for this project. I certify that said PWS is in compliance with applicable planning requirements in Rule 62-555.348, F.A.C.; applicable cross-connection control requirements in Rule 62-555.360, F.A.C.; and to the best of my knowledge and belief, all other applicable rules in Chapters 62-550, 62-555, and 62-699, F.A.C.; furthermore, I certify that, to the best of my knowledge and belief, said PWS's connection to this project will not cause said PWS to be in noncompliance with Chapter 62-550 or 62-555, F.A.C. I also certify that said PWS has reviewed the preliminary design report for this project and that said PWS considers the connection(s) between this project and said PWS acceptable as designed.

- Name(s) of Water Treatment Plant(s) to Which this Project Will Be Connected.

City of Port St. Joe

- Total Permitted Maximum Day Operating Capacity of Plant(s), gpd: \_\_\_\_\_
- Total Maximum Day Flow at Plant(s) as Recorded on Monthly Operating Reports During Past 12 Months, gpd: \_\_\_\_\_

\_\_\_\_\_  
Signature and Date

Jim Anderson

Printed or Typed Name

City Manager

Title

#### C. Certification by PWS that Will Own Project After It Is Placed into Permanent Operation

I am duly authorized to sign this notice on behalf of the PWS identified in Part I.H of this notice. I certify that said PWS will own this project after it is placed into permanent operation. I also certify that said PWS has reviewed the preliminary design report for this project and that said PWS considers this project acceptable as designed.

\_\_\_\_\_  
Signature and Date

Larry McClamma

Printed or Typed Name

Plant Manager

Title



# NOTICE OF INTENT TO USE THE GENERAL PERMIT FOR CONSTRUCTION OF WATER MAIN EXTENSIONS FOR PWSs

Project Name: Windward Beach North Townhome

Permittee: The St. Joe Company


## D. Certification by Professional Engineer(s) in Responsible Charge of Designing Project\*

I, the undersigned professional engineer licensed in Florida, am in responsible charge of designing this project. I certify that, to the best of my knowledge and belief, the design of this project complies with Chapter 62-555, F.A.C. I also certify that, to the best of my knowledge and belief, this project is not being designed to include any of the following construction work:

- construction of water mains conveying raw or partially treated drinking water;
- construction of drinking water treatment, pumping, or storage facilities or conflict manholes;
- construction of water mains in areas contaminated by low-molecular-weight petroleum products or organic solvents;
- construction of an interconnection between previously separate public water systems or construction of water mains that create a "new system" as described under subsection 62-555.525(1), F.A.C.; or
- construction of water mains that will remain dry following completion of construction.

(A specific construction permit is required for each project involving any of the above listed construction work.)

Signature, Seal, and Date:

  
10/25/22

Printed/Typed Name: Christopher Shortt, PE

License Number: 78424

Portion of Preliminary Design Report for Which Responsible:  
All

Signature, Seal, and Date:

Printed/Typed Name:

License Number:

Portion of Preliminary Design Report for Which Responsible:

\* Except as noted in paragraphs 62-555.520(3)(a) and (b), F.A.C., projects shall be designed under the responsible charge of one or more professional engineers (PEs) licensed in Florida. If this project is being designed under the responsible charge of one or more PEs licensed in Florida, Part III.D of this notice shall be completed by the PE(s) in responsible charge. If this project is not being designed under the responsible charge of one or more PEs licensed in Florida, Part III.D does not have to be completed.

SURVEYOR'S NOTES:

- [illegible]

#### A. SOURCE CONTROL POINTS

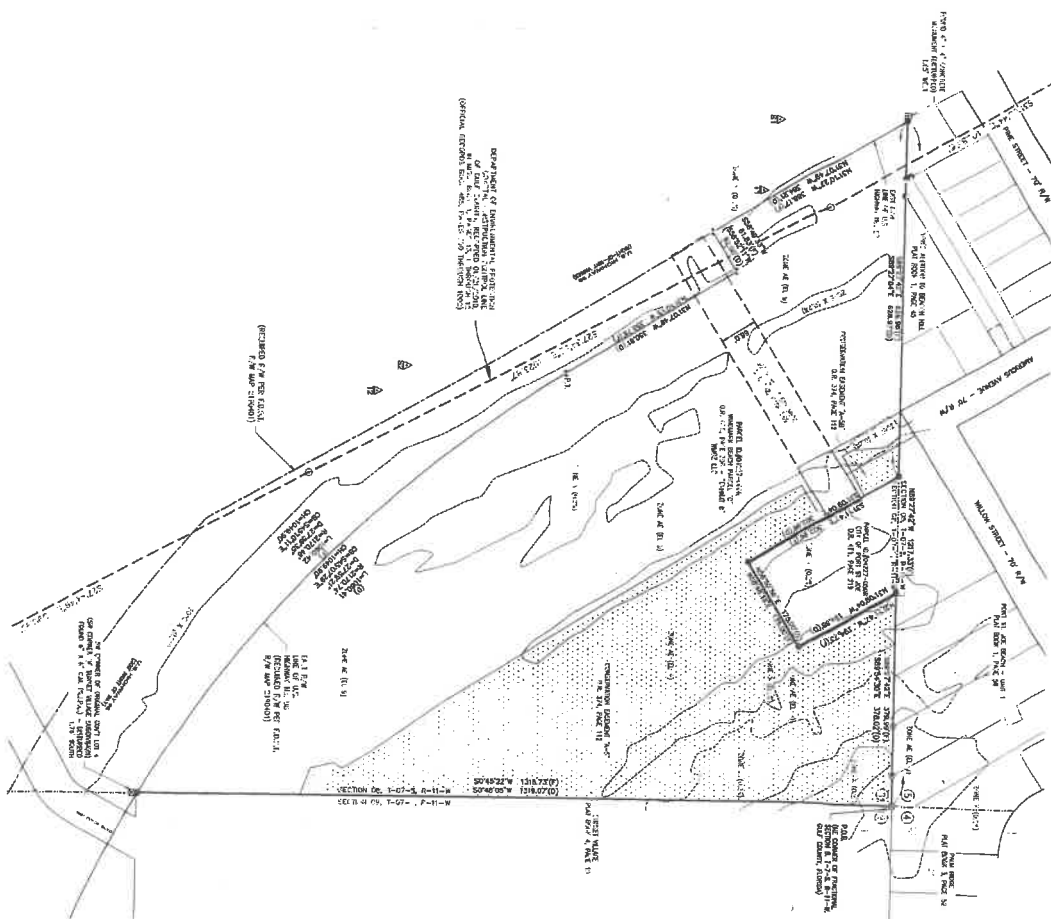
TELEGRAPH SITE CONTROL POINTS:		MAGNETIC	
TELEGRAPH SITE	COORDINATES	UTM	UTM
51. ROAD D.R. NUMBER 7-15	7-15	225050.00	11-37-41.9
52. ROAD TRAIL, CONTROL POINT 7-15	7-15	N/A	5.6
			11.74
53. ROAD TRAIL, CONTROL POINT 7-15	7-15		
54. ROAD TRAIL, CONTROL POINT 7-15	7-15		
55. ROAD TRAIL, CONTROL POINT 7-15	7-15		
56. ROAD TRAIL, CONTROL POINT 7-15	7-15		
57. ROAD TRAIL, CONTROL POINT 7-15	7-15		
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67. ROAD TRAIL, CONTROL POINT 7-15	7-15		
68. ROAD TRAIL, CONTROL POINT 7-15	7-15		
69. ROAD TRAIL, CONTROL POINT 7-15	7-15		
70. ROAD TRAIL, CONTROL POINT 7-15	7-15		

### TEMPORARY SITE CONTROL POINTS:

**LEGAL DESCRIPTION (WINDMILL BEACH - PARCEL "C")**  
D.E. BOOK 673, PAGE 3873

[illegible]

WINDMARK NORTH TOWNHOMES - SITE 1

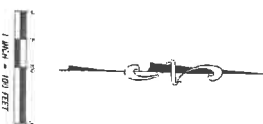


## SYMBOLS & ABBREVIATIONS

- [illegible]

INDEX OF SHEETS

- 2b. DATE RECORDED: DETAIL, CONTINUED  
\* NUMBER OF PAGES, AND LEGEND.
- 2c. SITE LOCATION DETAILS
- 2d-e. COPY NUMBER, SUBJECT, DETAIL.



REVISIONS		
DATE	REVISION	CHKD BY
10/24/2022	ADDED ADDITIONAL TOPO OF U.S. 68 FOR F.D.G.T. PERMIT	DB

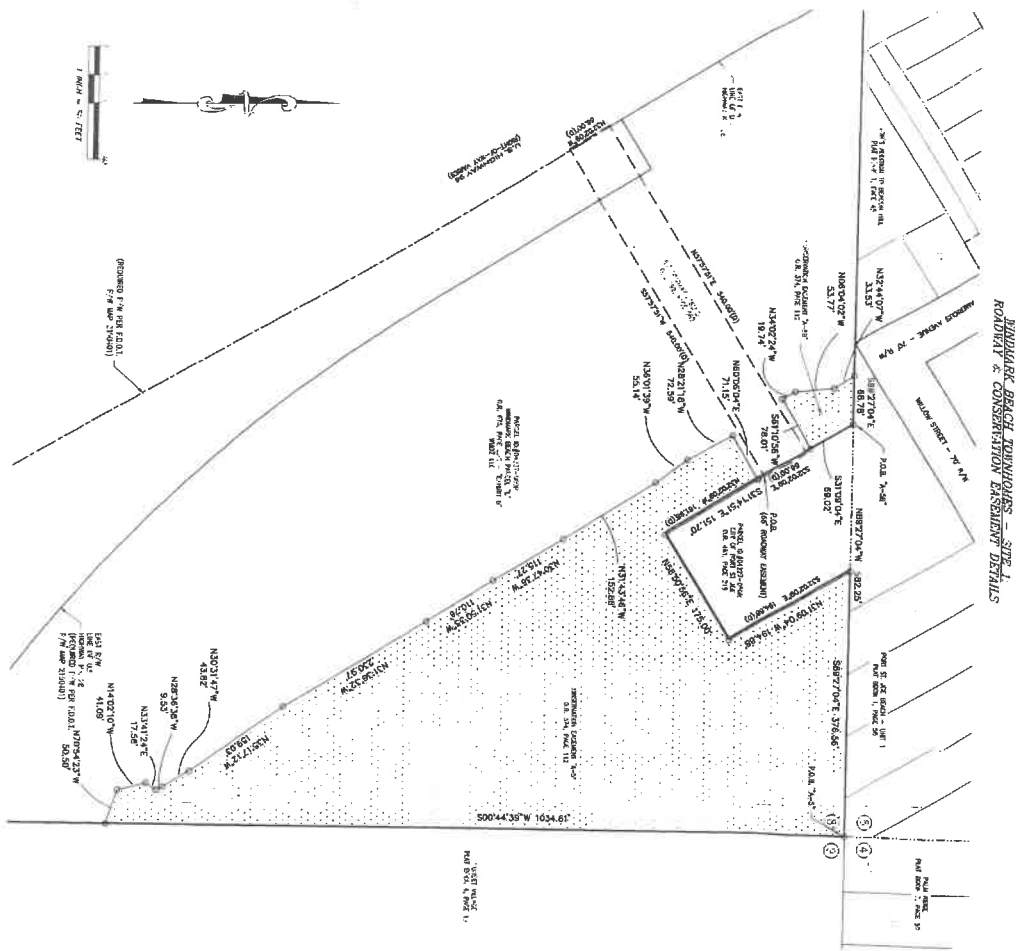
**Dewberry**  
DEWBERRY ENGINEERS INC.  
203 ABERDEEN PARKWAY  
PANAMA CITY, FLORIDA 32405  
PHONE: 850.522.0644 FAX: 850.522.1011  
WWW.DEWBERRY.COM  
CERTIFICATE OF AUTHORIZATION NO. LB 6011

DAVID JON BARTLETT, P.E.M. DATE SIGNED \_\_\_\_\_  
PROFESSIONAL LAND SURVEYOR No. 65402

BOUNDARY & TOPOGRAPHIC SURVEY  
SECTION 08, TOWNSHIP 07  
SOUTH, RANGE 11 WEST  
WINDMARK NORTH TOWNHOMES  
GULF COUNTY, FLORIDA

FB/PO	
204/23	
FIELD DATE	
01/11/2022	
DRAWING DATE	
01/18/2022	
BY	
RINO	
APPROVED	
QAB	
PROJECT NO.	
50144758	
SCALE	
1" = 100'	

5



**LIST OF SHEETS:**  
 1. SITE LOCATION MAP, GENERAL NOTES,  
 2. SITE EASEMENT DETAILS,  
 3. SITE EASEMENT DETAILS.

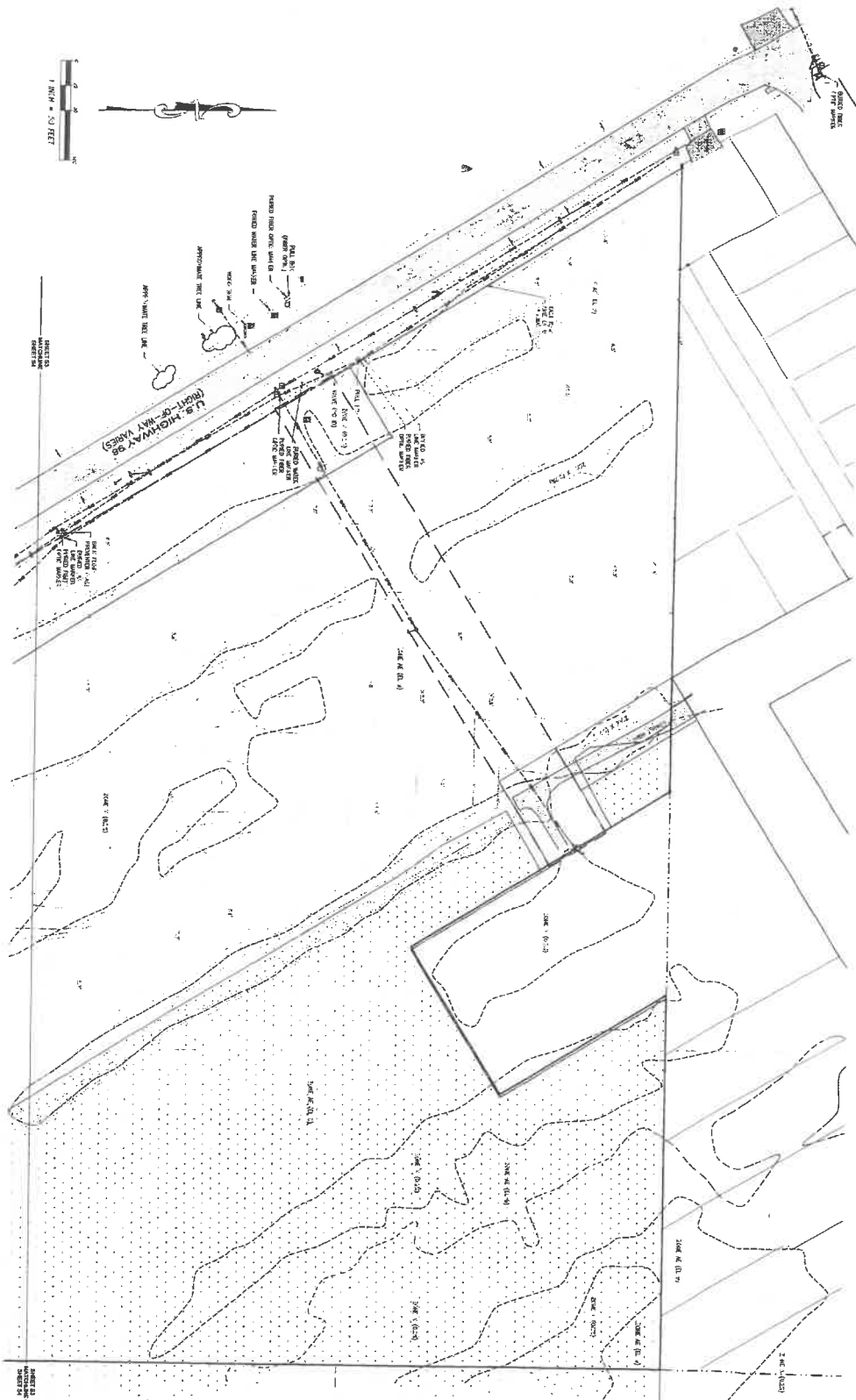
S2

BOUNDARY & TOPOGRAPHIC SURVEY  
 SECTION 08, TOWNSHIP 07  
 SOUTH, RANGE 11 WEST  
 WINDMARK BEACH TOWNHOMES  
 GULF COUNTY, FLORIDA

DAVID JON DAVENET, PLSM DATE: 10/24/2022  
 PROFESSIONAL LAND SURVEYOR NO. 12458

**Dewberry**  
 DEWBERRY ENGINEERS INC.  
 803 ANDERSON PARKWAY  
 PANAMA CITY, FLORIDA 32405  
 PHONE: 850.522.0844 FAX: 850.522.1611  
 WWW.DWBENGINEERS.COM  
 CERTIFICATE OF AUTHORIZATION NO. LB 8811

DATE	REVISION	CHKD
10/24/2022	ADDED ADDITIONAL TOPO OF U.S. WE FOR F.D.A.T. PERMIT	DB



WINDMARK BEACH TOWNHOMES - SITE 1  
 TOPOGRAPHIC SURVEY DETAILS

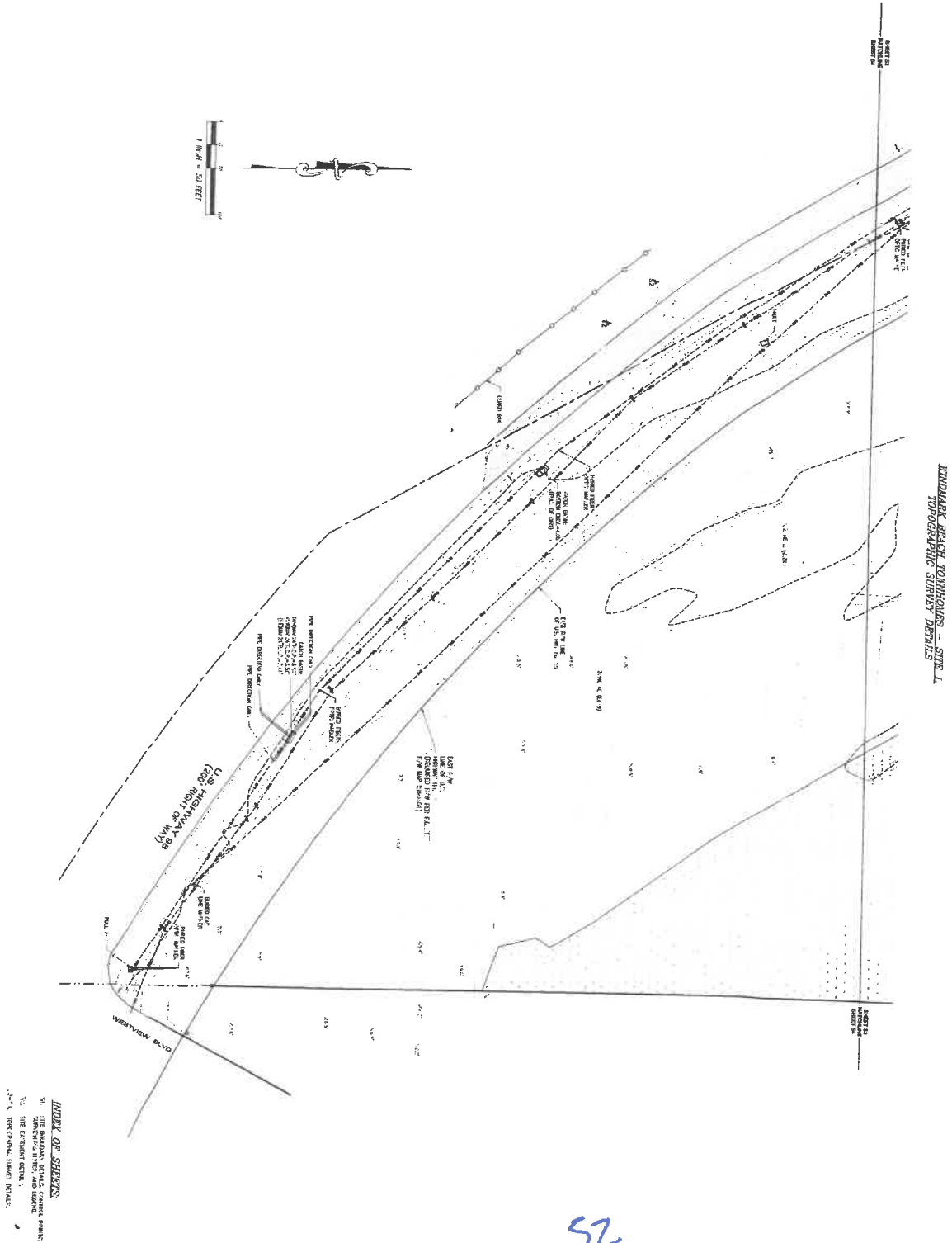
INDEX OF SHEETS  
 1. SITE PLAN  
 2. SITE ELEVATION  
 3. TOPOGRAPHIC SURVEY DETAILS

S3

NO.	DESCRIPTION
1	BOUNDARY & TOPOGRAPHIC SURVEY
2	SECTION 08, TOWNSHIP 07
3	SOUTH, RANGE 11 WEST
4	WINDMARK BEACH TOWNHOMES
5	GULF COUNTY, FLORIDA

**Dewberry**  
 DEWBERRY ENGINEERS INC.  
 250 ANDERSON PARKWAY  
 PANAMA CITY, FLORIDA 32405  
 PHONE: 904.232.8844 FAX: 904.232.1911  
 WWW.DEWBERRY.COM  
 CERTIFICATE OF AUTHORIZATION NO. LB 8011

DATE	REVISIONS	BY
10/24/2022	ADDED ADDITIONAL TOPO OF U.S. 98 FOR F.D.O.T. PERMIT	DE



**INDEX OF SHEETS**  
 NO. 1 - SITE ELEVATION, LOT AREA, CORNER POINTS,  
 TO: SITE ELEVATION DETAILS  
 NO. 2 - TOPOGRAPHIC SURVEY DETAILS

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S4

BOUNDARY & TOPOGRAPHIC SURVEY				REVISIONS		CHK	DB
SECTION 08, TOWNSHIP 07				DATE	REVISION		
SOUTH, RANGE 11 WEST				10/24/2022	ADDED ADDITIONAL TOPO OF U.S. 98 FOR F.D.O.T. POINT		
WINDMARK BEACH TOWNHOMES							
GULF COUNTY, FLORIDA							
BY: PDMARL	DATE: 10/24/2022	PROJECT NO.: 0144729	SCALE: 1" = 50'	<b>Dewberry</b> DEWBERRY ENGINEERS INC. 201 NORTHERN PARKWAY PANAMA CITY, FLORIDA 32405 PHONE: 904.822.0444 FAX: 904.822.1011 WWW.DEWBERRY.COM CERTIFICATE OF AUTHORIZATION NO. LB 8011			
CHAD EN BARETT, P.E. DATE: 10/24/2022 PROFESSIONAL LAND SURVEYOR NO. 15493							



# WINDMARK BEACH NORTH TOWNHOMES

## STORMWATER MANAGEMENT REPORT

**Prepared for:**



**THE ST. JOE COMPANY  
130 RICHARD JACKSON BLVD, SUITE 200  
PANAMA CITY BEACH, FL 32407**

**Prepared By:**



**OCTOBER 2022**

**PROJECT NO. 50144759**

**DEWBERRY**  
203 ABERDEEN PARKWAY  
PANAMA CITY, FLORIDA 32405  
PHONE: 850.522.0644 / FAX: 850.522.1011

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## Professional Engineer's Certification

I hereby certify that I am a Licensed Professional Engineer in the State of Florida practicing with Dewberry and that I have supervised the preparation of and approve the evaluations, findings, opinions, conclusions, and technical advice hereby reported for:

**Project:** Windmark North Townhomes  
Stormwater Management Report  
Dewberry Project No. 50144759

**Location:** Port St. Joe, Florida 32456

---

Jonathan Sklarski, P.E.  
Associate Vice President  
License No. 67361

---

Date

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### Supporting Documentation

EXHIBIT 1 – GIS EXHIBITS  
EXHIBIT 2 – NRCS SOILS REPORT  
EXHIBIT 3 – SWMF CALCULATIONS  
EXHIBIT 4 – ICPR INPUT DATA AND RESULTS  
EXHIBIT 5 – DRAINAGE BASIN MAPS  
EXHIBIT 6 – GEOTECHNICAL REPORT  
EXHIBIT 7 – STORMWATER POND OPERATION AND MAINTENANCE PLAN

**ENGINEER'S NARRATIVE  
STORMWATER MANAGEMENT REPORT  
WINDMARK BEACH NORTH TOWNHOMES  
PORT ST. JOE, FLORIDA**

**INTRODUCTION**

The proposed project is the construction of a new 78 lot townhome subdivision, with associated roads, utilities, and a stormwater management system. The project is east of US Highway 98 in Gulf County, FL. Please see **Exhibit 1** for the Project Location Map. Tax parcel ID for the site parcel is:

Parcel: 04227-000R (19.58-ac)

The disturbed project area is situated on approximately 8.86 acres of the above parcel. The site is currently undeveloped and is comprised mostly of flat woodlands. Please see **Sheet C3** of the Engineering Plans for the wetland locations. No new wetland impacts are proposed in this development. A previously permitted outfall location into the wetland system is utilized for stormwater maintenance facility outfall.

**STORMWATER MANAGEMENT OVERVIEW**

**Pre-Developed Conditions**

The existing site is an undeveloped flat woodland with pine trees, dense undergrowth, and sandy trails winding throughout. The soils at this site consist of light gray clean fine sands. In the natural state of the site, the stormwater runoff flows to the wetland system northeast of the project. Please see **Exhibit 5** for the Pre-Development Drainage Basins Map. The natural flow of the stormwater runoff will be considered for pre-versus post-development attenuation for each basin. The drainage area for the project's Pre-Development Basin 1 is approximately 5.92 acres. The stormwater runoff that drains to the northeast wetland system was used to establish the pre-development basin.

**Post-Development Conditions**

Stormwater runoff from the site will be collected and routed via a series of inlets and pipes and routed to a proposed wet detention stormwater management facility (swmf). The stormwater runoff will be treated by one (1) wet detention stormwater management facility (SWMF #1). The runoff from the project area is treated and attenuated before it is discharged. Once water quality and attenuation criteria are met, the outfall will discharge at a controlled rate and overland flow into the wetland system located to the northeast of the project.

### **Wet Detention Stormwater Management Facility #1 (SWMF #1)**

SWMF #1 is designed to treat 8.86-acres of the disturbed limits. An FDOT Type C Inlet structure is proposed in SWMF #1 and will serve as the outfall structure for this facility. Discharge from the outfall structure will be routed via piping to the north of the facility to a bubble up structure surrounded by rip rap to dissipate discharge energy. The discharge will then flow into the wetland system located to the northeast of the project and ultimately flow to the Gulf of Mexico. The stormwater management facility has been designed to recover ½ the treatment volume within 48 to 60 hours with a drawdown orifice. The pond has also been designed to route each storm event without overtopping.

### **STORMWATER MODELING TECHNIQUES**

Interconnected Channel and Pond Routing (ICPR version 4.07.08) was used as the stormwater management facility (SWMF) modeling software. This program has been accepted by numerous regulatory agencies across Florida and is accepted by FEMA for Flood Insurance projects. ICPR uses a link-node concept in conjunction with the basin data to idealize real-world systems. A node is a discrete location in the drainage system where conservation of mass or continuity is maintained. Links are the connections between nodes and are used to transfer or convey water through the drainage conveyance system. The input data, network diagram, and node maximum conditions (maximum stage elevation) are included with this report.

### **DESIGN CRITERIA**

#### **Water Quality:**

#### **Wet Detention Systems:**

It is an ERP and Port St. Joe requirement to provide treatment of one inch of rainfall over the contributing area, when utilizing wet detention stormwater management facilities. (Part V, Section 8.2 of the ERP Applicant's Handbook Volume II).

**Flood Attenuation:** The site was designed in accordance with ERP and Port St. Joe requirements for flood attenuation. The SWMF has been designed to attenuate the 2-year/24-hour SCS III storm event per ERP requirement. The SWMF has also been designed to attenuate the 25-year/24-hour storm event to meet Port St. Joe attenuation criteria. FDOT and SCS III hydrographs were modeled to determine the pre-development and post-development runoff rates for these events.

## GEOTECHNICAL INFORMATION

Geotechnical information was provided by Magnum Engineering, Inc. on November 18, 2021, to determine the seasonal high-water table elevations and subsurface conditions for the subject project area. A copy of the Geotechnical Report is included in **Exhibit 6** of this report. This geotechnical report contained information needed in the design of an effective pavement and stormwater system for the development area. As part of the geotechnical services ten (10) 5-ft deep hand auger borings were performed in the proposed roadway and the proposed stormwater pond locations. The locations of these borings are marked on the boring location map included in the attached Geotechnical Report (**Exhibit 6**). Based on the enclosed geotechnical information, the groundwater table was estimated to be between 0-ft and <5-ft below existing grade on the project site.

### Wet Stormwater Management Facility #1 (SMWF #1)

The seasonal high groundwater table (SHGWT) used for establishing the permanent pool of the proposed stormwater management facility was determined by HA-8, HA-9, and HA-10. This hand auger estimated the SHGWT to be approximately at existing grade, resulting in a design permanent pool elevation of 7.00. Please reference the Geotechnical Report included in **Exhibit 6**.

## POST DEVELOPMENT TREATMENT ANALYSIS

### Wet Detention Stormwater Management Facility #1 (SWMF #1)

Per ERP Applicant's Handbook Volume II, Section 8.2, the treatment volume required is:

Treatment of 1.0" of rainfall over the basin area

$$TV_{reqd} = 8.86 \text{ acres (basin)} \times 43,560 \frac{ft^2}{acres} \times 1.0 \text{ in} \times \frac{1 \text{ ft}}{12 \text{ in}} = 32,176 \text{ ft}^3$$

The proposed control elevation of the wet SWMF is 7.00-ft. Utilizing the Wet Pond Calculations Spreadsheet (**Exhibit 3**) the weir invert elevation has been set at 7.50-ft which provides a treatment volume of 35,452 ft<sup>3</sup>. For post-development basin delineations see **Exhibit 5**. The drainage basin for SWMF 1 is 8.86-acres. A 10-minute time of concentration was utilized in the model to represent the basin drainage. The 2-year/24-hour SCS III storm event had a post-development rate of 3.38 cfs. The 25-year/24-hour had a post-development rate of 5.23 cfs. The appropriate land use curve numbers and land uses areas were input into each basin in ICPR (see ICPR Input Data and Results in **Exhibit 4** for curve number input data). The outfall will control the treatment and rate of discharge. The following table contains design information for the stormwater management facilities:



<b>Wet Detention SWMF 1</b>	Top of Bank Elevation:	9.00 ft NAVD88
	Pond Bottom Elevation:	-5.00 ft NAVD88
	Pond Control Elevation:	7.00 ft NAVD88
	Req'd Treatment Volume:	32,176 ft <sup>3</sup>
	Provided Treatment Volume:	35,452 ft <sup>3</sup>
	Overflow Weir Invert Elev. (Two 1.08' H x 1.50' W Slot):	7.50 ft NAVD88
	Max Pond Stage Elevation:	8.60 ft NAVD88 (100yr - 8hr)
	Drawdown Recovery Time:	53 hours

## UNCAPTURED AREAS

The drainage basin for SWMF 1 is 8.86 acres with 0.88 acres being uncaptured area.

This uncaptured area is the result of pervious tie in grading along the perimeter of the site. This uncaptured area has also been routed directly to the pond outfall node in ICPR, so its discharge rate is accounted for in the attenuation analysis.

## PRE-DEVELOPMENT AND POST DEVELOPMENT RUNOFF RATES SWMF 1 & SWMF 2

Storm Event	Pre-Development Runoff Rate (cfs)	Post-Development Runoff Rate (cfs)
002 YR - 024 HR SCS III	<b>11.21</b>	<b>3.38</b>
025 YR - 001 HR	11.87	1.41
025 YR - 002 HR	12.27	3.72
025 YR - 004 HR	11.05	5.78
025 YR - 008 HR	12.86	7.21
025 YR - 024 HR	<b>5.45</b>	<b>5.23</b>
100 YR - 001 HR	15.23	3.17
100 YR - 002 HR	15.88	6.18
100 YR - 004 HR	14.43	8.86
100 YR - 008 HR	16.60	9.75
100 YR - 024 HR	6.98	7.34

## EROSION CONTROL

Best Management Practices for erosion control measures will be utilized throughout the construction phase of this project to protect the existing conditions and to restrict any turbid runoff from entering or leaving the construction site. Temporary stabilization may be utilized as determined necessary. All drainage inlets shall be protected utilizing inlet sediment barriers. Installation details for silt fences, tree barriers and hay bales, where applicable, are

included within this permit application. Please see the Erosion Control Plans in the plan set for specific details.

## **CONCLUSION**

The design of the stormwater management system for this project complies with the following requirements: FDEP ERP, Port St. Joe

The development exceeds the ERP and Port St. Joe requirement to provide treatment of one inch of rainfall over the contributing area for wet detention stormwater management facilities.

The proposed design meets the ERP and Port St. Joe recovery requirement of half of the treatment volume within 48 to 60 hours of the storm event for wet detention facilities.

The proposed design meets the ERP attenuation requirement for stormwater runoff caused by the 2-year/24-hour SCS III storm event.

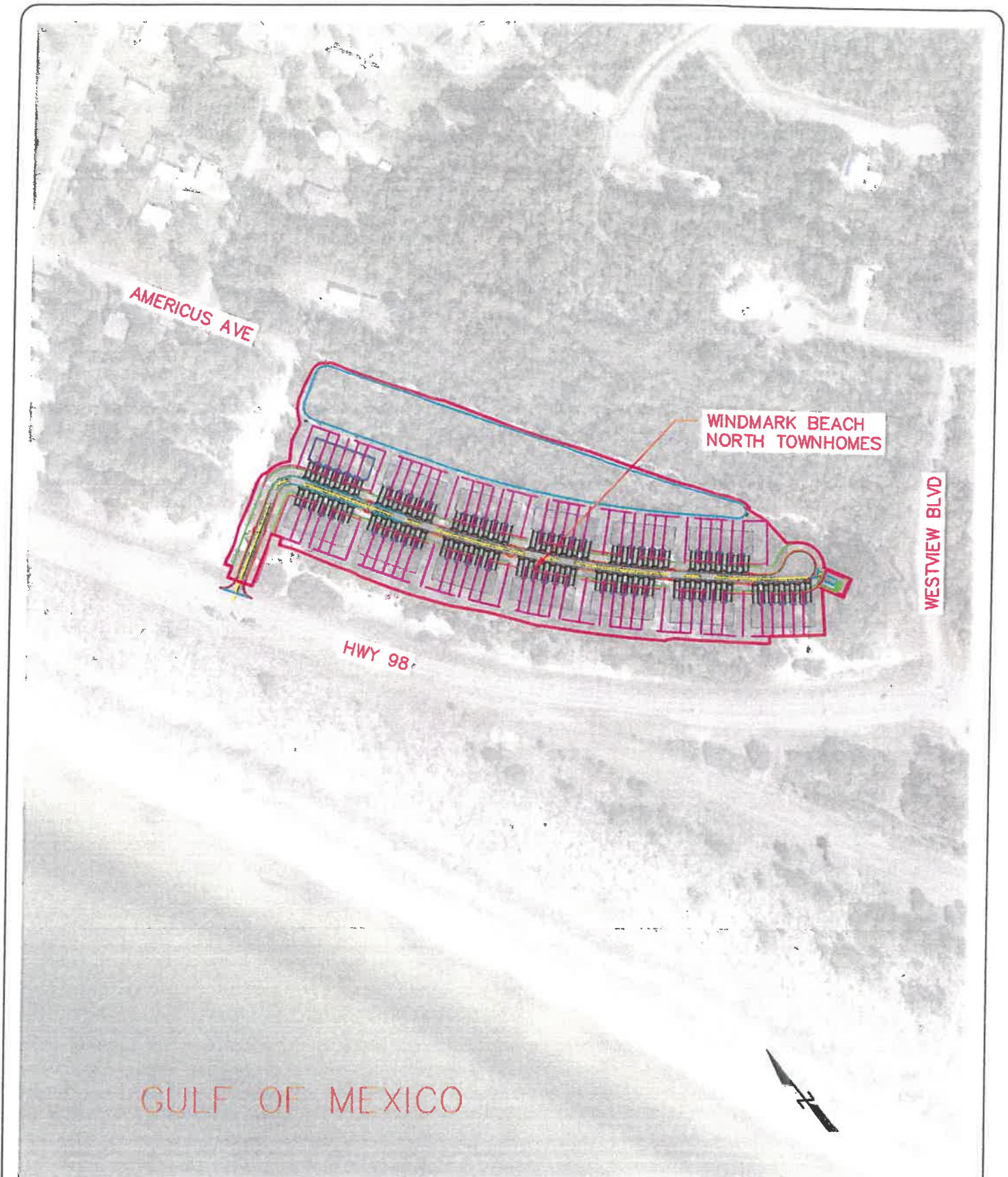
The proposed design also meets Port St. Joe criteria for attenuation of the 25-year/24-hour storm event.

## **FDEP NPDES**

BMP erosion control measures will be utilized throughout the construction phase of this project to restrict any turbid runoff from entering or leaving the construction site. It is the Contractor's responsibility to obtain the NPDES permit and provide appropriate permit administration. Installation details for silt fences and inlet sediment barriers, where applicable, are included within this permit application.

# EXHIBIT 1

## GIS EXHIBITS



September 16, 2022 (08:49:26 EST)  
 K:\50144759\_SJC\_WINDMARK N. TOWNHOMES\ENGINEERING\STORMWATER CALCS\CAD\WINDMARK NORTH TOWNHOMES PARCELS EXHIBIT.DWG 1 - PL. BY: EEVANS

EB# 0006794



**Dewberry®**

203 Aberdeen Parkway, Panama City, FL 32405  
 850.522.0644, www.dewberry.com

### PROJECT LOCATION

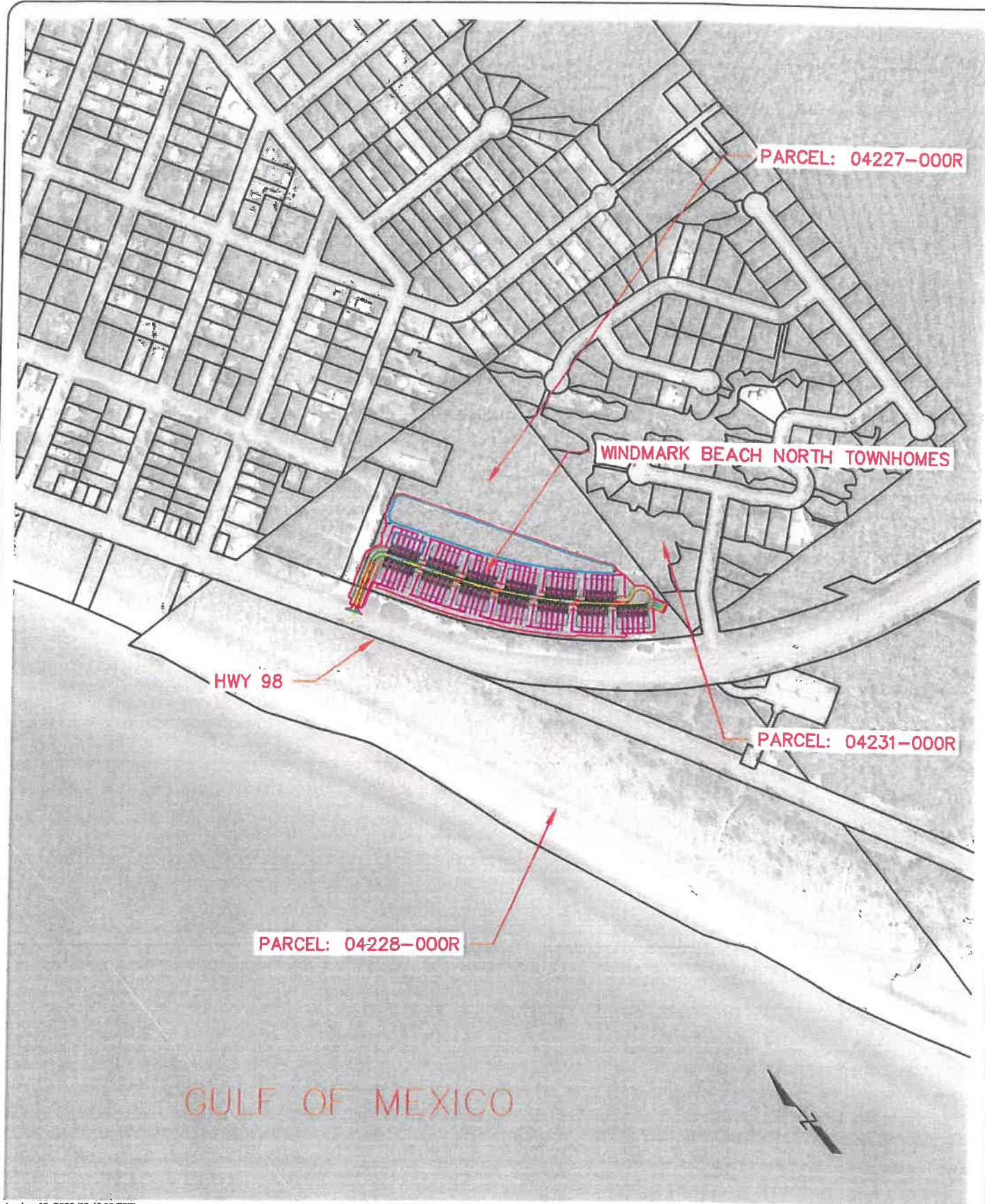
WINDMARK BEACH NORTH  
 TOWNHOMES  
 GULF COUNTY, FLORIDA

DATE:  
 SEPTEMBER 2022  
 SCALE:  
 1" = 250'  
 DRAWN:  
 EE  
 CHECKED:  
 JS

PROJECT NO.  
 50144759  
 SHEET  
 1

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September 16, 2022 (08:49:26 EST)  
 K:\50144759 SJC WINDMARK N. TOWNHOMES\ENGINEERING\STORMWATER CALC\SCAD\WINDMARK NORTH TOWNHOMES PARCELS EXHIBIT.DWG 2 - PARCELS BY: EEVANS

ED# 0008794



203 Aberdeen Parkway, Panama City, FL 32405  
 850.522.0644, www.dewberry.com

### PARCELS

WINDMARK BEACH NORTH  
 TOWNHOMES  
 GULF COUNTY, FLORIDA

DATE:  
 SEPTEMBER 2022  
 SCALE:  
 1" = 250'  
 DRAWN:  
 EE  
 CHECKED:  
 JS

PROJECT NO.  
 50144759  
 SHEET  
 2

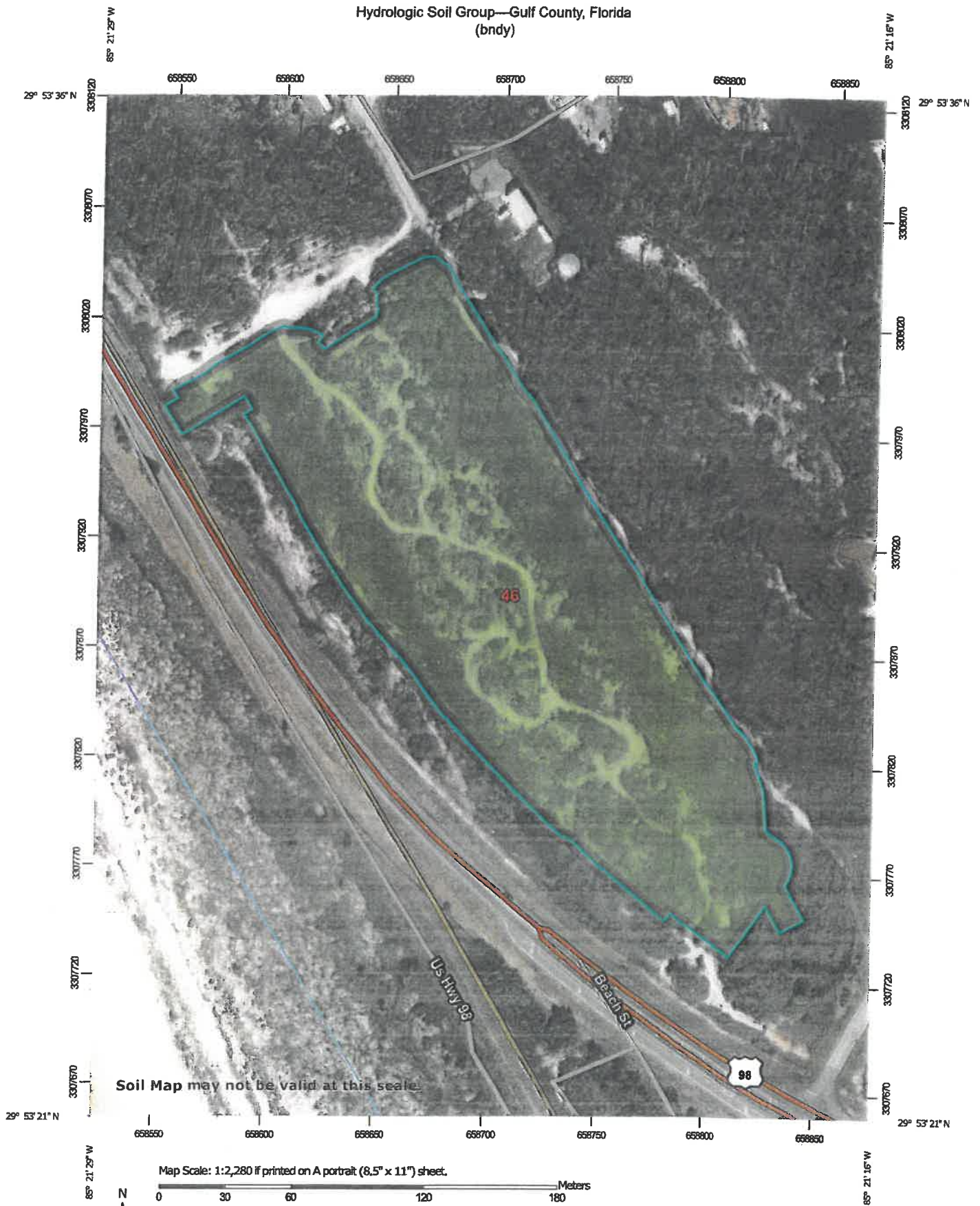
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## EXHIBIT 2

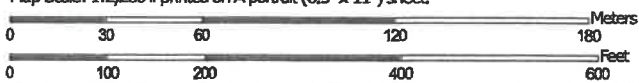
# NRCS SOILS REPORT

# Hydrologic Soil Group—Gulf County, Florida (bndy)



Soil Map may not be valid at this scale.

Map Scale: 1:2,280 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 16N WGS84



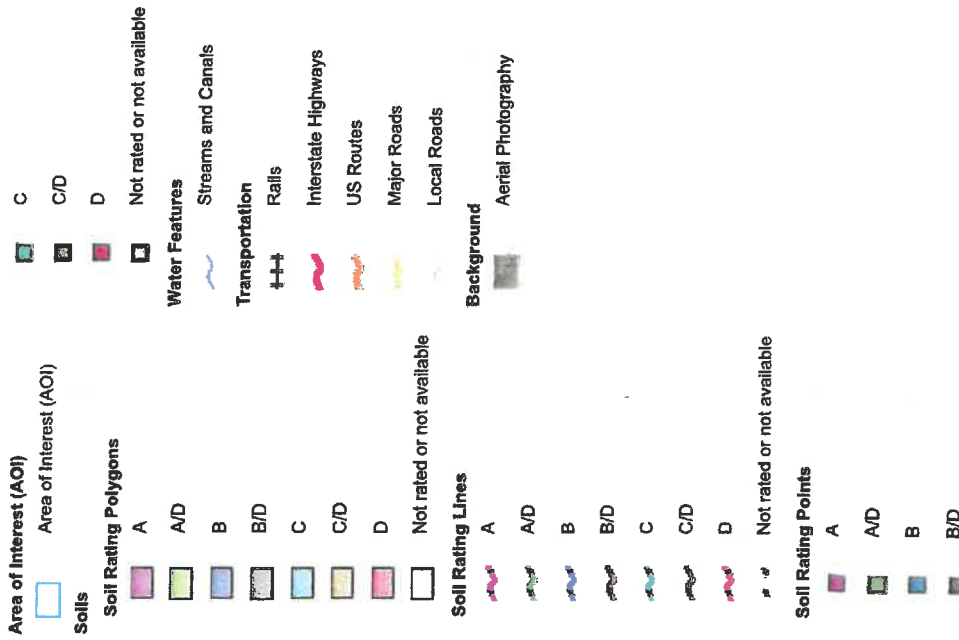
Natural Resources  
Conservation Service

Web Soil Survey  
National Cooperative Soil Survey

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9/1/2022  
Page 1 of 4

## MAP LEGEND



## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Gulf County, Florida  
Survey Area Data: Version 18, Sep 7, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 2, 2020—Dec 8, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
46	Corolla-Duckston complex, gently undulating, flooded	A/D	8.7	100.0%
Totals for Area of Interest			8.7	100.0%

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

**Group A.** Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

**Group B.** Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

**Group C.** Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

**Group D.** Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### Rating Options

*Aggregation Method:* Dominant Condition

*Component Percent Cutoff: None Specified*

*Tie-break Rule: Higher*



## EXHIBIT 3

# SWMF CALCULATIONS

**WINDMARK BEACH NORTH TOWNHOMES  
WET POND DESIGN  
SWMF 1**

**BASIN DATA:**

**Post Development Basin Data:**

Impervious Area	3.66	Acres
Uncaptured Area	0.88	Acres
Grass/Natural Area=	2.40	Acres
Pond Area=	1.92	Acres
Offsite Area (CN based on existing land use) =	0.00	Acres
Total Project Area=	8.86	Acres

Runoff Coefficient	Curve Number
0.95	98
0.25	39
0.25	39
1.00	100
0.32	49
0.70	77

**STORMWATER POND DATA**

elevation (FT.)	TOTAL (SF.)	area (ac.)	avg. area (SF.)	delta H (FT.)	volume (CF)	sum vol. (CF)	sum vol. CY	NOTATIONS
-5.0	18260	0.4192	0	0	0	0	0	
-4.0	20885	0.4794	19572	1.00	19572	19572	726	
-3.0	23611	0.5420	22248	1.00	22248	41820	1549	
-2.0	26436	0.6069	25023	1.00	25023	66844	2476	
-1.0	29354	0.6739	27895	1.00	27895	94738	3509	
0.0	32368	0.7431	30861	1.00	30861	125599	4662	
1.0	35481	0.8145	33925	1.00	33925	159524	5908	
2.0	38689	0.8862	37065	1.00	37065	196609	7282	
3.0	41985	0.9638	40337	1.00	40337	236946	8776	
4.0	45348	1.0069	42967	1.00	42967	279912	10367	
5.0	52418	1.2033	48183	1.00	48183	328095	12152	
6.0	59731	1.3712	56074	1.00	56074	384170	14229	
7.0	67147	1.5415	63439	1.00	63439	447609	16578	Permanent Pool
8.0	74661	1.7140	70904	1.00	70904	518513	19204	
9.0	82275	1.8888	78468	1.00	78468	596981	22110	

Treatment Volume Required (1.0" over Drainage Basin)=	32176	cf
Treatment Volume Provided @ Elev	7.45	ft
Treatment Volume Set @ Elev	7.50	ft
Treatment Volume @ Treatment Elevation	35452	cf

**PERMANENT POOL VOLUME**

28.5 inches/122 days - 14 day residence time during rainy period of year (122 days)

Coef. of Runoff=

0.70 Drainage Area(sf)=

386,116 sf

Volume=(c)(Area in sf)(avg. rainfall in feet)(14 d/122 d)=

Permanent Pool Required =	73,810	cf
Permanent Pool Req'd Non-littoral =	110,716	cf
Permanent Pool Provided =	447,609	cf
Mean Permanent Pool Depth =	6.67	ft

**BLEED DOWN ORIFICE**

No more than 1/2 the treatment volume can discharge within the first 48-60 hours.

1/2 Treatment Volume:

16,088 cf

Discharge rate:

0.0745 cfs (@ T=60hrs)

0.0931 cfs (@ T=48hrs)

Orifice equation :

$Q=C \times A \times (\text{sqrt. of } 2 \times g \times h)$

C=

0.6

Q=

0.07 cfs (T=60hrs)

0.09 cfs (T=48hrs)

g=

32.2 ft/s<sup>2</sup>

average h=

$(h_1+h_2)/2$

0.39 ft

Solving for Area =

0.0248 sf

0.0310 sf

diameter=

2.13 inches (T=60hrs)

2.38 inches (T=48hrs)

USE 2.25 DIA ORIFICE

## EXHIBIT 4

### ICPR INPUT DATA AND RESULTS

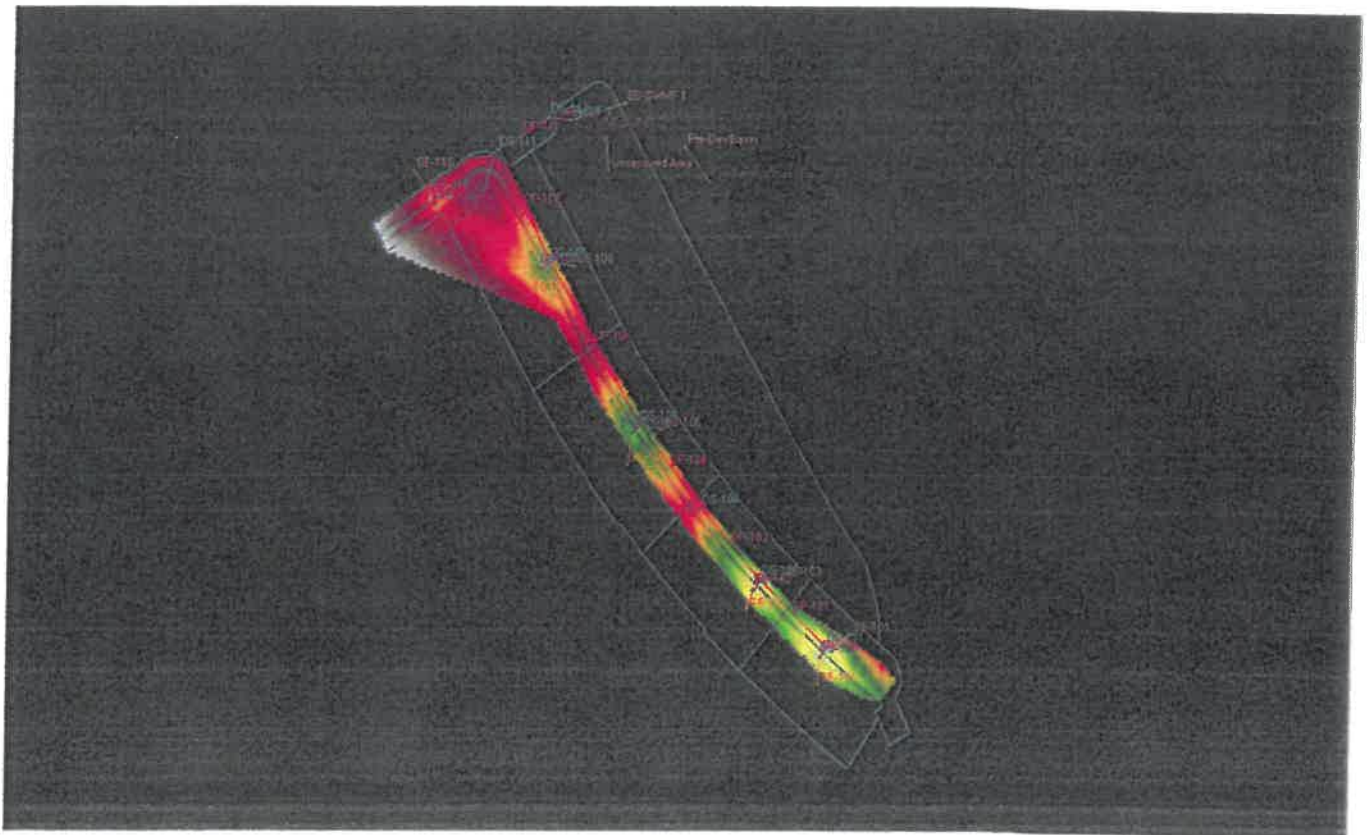
# Windmark Beach North Townhomes

ICPR INPUT DATA & RESULTS

Dewberry Project Number: 50144759

(September 2022)

Node Diagram



Simulation: 002-074 (SCS III)

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:49:15 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	32.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

## Output Time Increments

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

## Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

## Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

## Resources &amp; Lookup Table

Save Restart: False

## Resources

Rainfall Folder:

Reference ET Folder:

Unit Hydrograph Folder:

## Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

K:\5014759\_8JC\_Windmark N. Townhomes\Engineering\Stormwater Calc\ICPHDesign\ICPH4

9/18/2022 13:17

Conductivity Set:

Leakage Set:

## Variables &amp; Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Dft Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 100 R2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~SCSIII-24

Rainfall Amount: 6.40 in

Storm Duration: 24.0000 hr

Dft Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 100 R2

Energy Switch (1D): Energy

Comment:

Simulation: 025-041

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:49:59 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	4.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		60.0000	

## Output Time Increments

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

K:\5014759\_8JC\_Windmark N. Townhomes\Engineering\Stormwater Calc\ICPHDesign\ICPH4

9/18/2022 13:17

## Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

## Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	360.0000

## Resources &amp; Lookup Table

Save Restart: False

## Resources

Rainfall Folder: ICPH3

Reference ET Folder:

Unit Hydrograph Folder:

## Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

Conductivity Set:

Leakage Set:

## Variables &amp; Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Dft Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 1 R2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~FDOT-1

Rainfall Amount: 3.80 in

Storm Duration: 1.0000 hr

Dft Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 113 R2

Energy Switch (1D): Energy

Comment:

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9/18/2022 13:17

Simulation: 025-042

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:50:06 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	6.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		60.0000	

## Output Time Increments

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	360.0000

## Resources &amp; Lookup Table

Save Restart: False

## Resources

Rainfall Folder: ICPH3

Reference ET Folder:

Unit Hydrograph Folder:

## Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

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Conductivity Set:  
Leakage Set:

### Tolerances & Options

Time Marching: SAOR  
Max Iterations: 6  
Over-Relax Weight: 0.5 dec  
Fact:  
dZ Tolerance: 0.0010 ft  
Max dZ: 1.0000 ft  
Link Optimizer Tol: 0.0001 ft  
Edge Length Option: Automatic  
Dft Damping (2D): 0.0050 ft  
Min Node Srf Area: 1 ft2  
(2D):  
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False  
Smp/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-2  
Rainfall Amount: 5.00 in  
Storm Duration: 2.0000 hr  
Dft Damping (1D): 0.0050 ft  
Min Node Srf Area: 113 ft2  
(1D):  
Energy Switch (1D): Energy

Comment:

Simulation: 025-034

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:50:15 PM  
Program Version: ICPM 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	10.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
60.0000	60.0000	

### Output Time Intervals

#### Hydrology

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

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### Surface Hydraulics

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

### Groundwater

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	360.0000

Save Restart: False

### Resources & Lookup Tables

Rainfall Folder: ICPM3  
Reference ET Folder:  
Unit Hydrograph Folder: ICPM3

Lookup Tables  
Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1  
Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:  
Conductivity Set:  
Leakage Set:

### Tolerances & Options

Time Marching: SAOR  
Max Iterations: 6  
Over-Relax Weight: 0.5 dec  
Fact:  
dZ Tolerance: 0.0010 ft  
Max dZ: 1.0000 ft  
Link Optimizer Tol: 0.0001 ft  
Edge Length Option: Automatic  
Dft Damping (2D): 0.0050 ft  
Min Node Srf Area: 1 ft2  
(2D):  
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False  
Smp/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-4  
Rainfall Amount: 6.00 in  
Storm Duration: 4.0000 hr  
Dft Damping (1D): 0.0050 ft  
Min Node Srf Area: 113 ft2  
(1D):  
Energy Switch (1D): Energy

Comment:

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9/19/2022 13:17

Simulation: 025-035

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:50:25 PM  
Program Version: ICPM 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	20.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
60.0000	60.0000	

### Output Time Intervals

#### Hydrology

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

#### Surface Hydraulics

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

#### Groundwater

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	360.0000

#### Resources & Lookup Tables

Save Restart: False

Rainfall Folder: ICPM3  
Reference ET Folder:  
Unit Hydrograph Folder: ICPM3

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:

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Conductivity Set:  
Leakage Set:

### Tolerances & Options

Time Marching: SAOR  
Max Iterations: 6  
Over-Relax Weight: 0.5 dec  
Fact:  
dZ Tolerance: 0.0010 ft  
Max dZ: 1.0000 ft  
Link Optimizer Tol: 0.0001 ft  
Edge Length Option: Automatic  
Dft Damping (2D): 0.0050 ft  
Min Node Srf Area: 1 ft2  
(2D):  
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False  
Smp/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-8  
Rainfall Amount: 7.60 in  
Storm Duration: 8.0000 hr  
Dft Damping (1D): 0.0050 ft  
Min Node Srf Area: 113 ft2  
(1D):  
Energy Switch (1D): Energy

Comment:

Simulation: 025-036

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:50:49 PM  
Program Version: ICPM 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	30.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
60.0000	60.0000	

### Output Time Intervals

#### Hydrology

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

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Simulation: 10/20/20

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:51:24 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

Start Time: Year 0, Month 0, Day 0, Hour [hr] 0.0000

End Time: Year 0, Month 0, Day 0, Hour [hr] 4.0000

Hydrology [sec] 60.0000

Surface Hydraulics [sec] 0.1000

Groundwater [sec] 900.0000

Min Calculation Time: 60.0000

Max Calculation Time: 60.0000

Output Time Intervals

Hydrology

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Surface Hydraulics

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Groundwater

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 360.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder: ICPH3

Reference ET Folder:

Unit Hydrograph Folder: ICPH3

Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Drift Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 1 ft2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~FDOT-24

Rainfall Amount: 10.80 in

Storm Duration: 24.0000 hr

Drift Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 113 ft2

Energy Switch (1D): Energy

Comment:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Drift Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 1 ft2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~FDOT-1

Rainfall Amount: 4.50 in

Storm Duration: 1.0000 hr

Drift Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 113 ft2

Energy Switch (1D): Energy

Comment:

Simulation: 9/19/2022

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:51:31 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

Start Time: Year 0, Month 0, Day 0, Hour [hr] 0.0000

End Time: Year 0, Month 0, Day 0, Hour [hr] 6.0000

Hydrology [sec] 60.0000

Surface Hydraulics [sec] 0.1000

Groundwater [sec] 900.0000

Min Calculation Time: 60.0000

Max Calculation Time: 60.0000

Output Time Intervals

Hydrology

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Simulation: 10/20/20

Scenario: Scenario1

Run Date/Time: 9/19/2022 12:51:24 PM

Program Version: ICPH 4.07.08

Run Mode: Normal

Start Time: Year 0, Month 0, Day 0, Hour [hr] 0.0000

End Time: Year 0, Month 0, Day 0, Hour [hr] 4.0000

Hydrology [sec] 60.0000

Surface Hydraulics [sec] 0.1000

Groundwater [sec] 900.0000

Min Calculation Time: 60.0000

Max Calculation Time: 60.0000

Output Time Intervals

Hydrology

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Surface Hydraulics

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Groundwater

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 360.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder: ICPH3

Reference ET Folder:

Unit Hydrograph Folder: ICPH3

Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

Tolerances & Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Drift Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 1 ft2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~FDOT-24

Rainfall Amount: 10.80 in

Storm Duration: 24.0000 hr

Drift Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 113 ft2

Energy Switch (1D): Energy

Comment:

Surface Hydraulics

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 15.0000

Groundwater

Year 0, Month 0, Day 0, Hour [hr] 0.0000

Time Increment [hr] 360.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder: ICPH3

Reference ET Folder:

Unit Hydrograph Folder: ICPH3

Lookup Tables

Boundary Stage Set:

Extern Hydrograph Set:

Curve Number Set: 1

Green-Ampt Set:

Vertical Layers Set:

Impervious Set: 1

Roughness Set:

Crop Coef Set:

Fillable Porosity Set:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching: SAOR

Max Iterations: 6

Over-Relax Weight: 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft

Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Drift Damping (2D): 0.0050 ft

Min Node Srf Area (2D): 1 ft2

Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr

ET for Manual Basins: False

Smp/Man Basin Rain Opt: Global

OF Region Rain Opt: Global

Rainfall Name: ~FDOT-2

Rainfall Amount: 6.00 in

Storm Duration: 2.0000 hr

Drift Damping (1D): 0.0050 ft

Min Node Srf Area (1D): 113 ft2

Energy Switch (1D): Energy

Comment:

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**Simulation: 100-013**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:51:39 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	10.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:

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**Simulation: 100-013**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:51:51 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	20.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:

Conductivity Set:  
Leakage Set:

IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False

Snip/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-4  
Rainfall Amount: 7.40 in  
Storm Duration: 4.0000 hr

Drift Damping (2D): 0.0050 ft  
Min Node Srf Area (2D): 1 ft2  
Energy Switch (2D): Energy

Drift Damping (1D): 0.0050 ft  
Min Node Srf Area (1D): 1 ft2  
Energy Switch (1D): Energy

Comment:

**Simulation: 100-013**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:51:51 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	20.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:

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**Simulation: 100-024**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:52:18 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	30.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:  
Conductivity Set:  
Leakage Set:

**Simulation: 100-024**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:52:18 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	30.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:  
Conductivity Set:  
Leakage Set:

IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False

Snip/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-8  
Rainfall Amount: 9.36 in  
Storm Duration: 8.0000 hr

Drift Damping (2D): 0.0050 ft  
Min Node Srf Area (2D): 1 ft2  
Energy Switch (2D): Energy

Drift Damping (1D): 0.0050 ft  
Min Node Srf Area (1D): 1 ft2  
Energy Switch (1D): Energy

Comment:

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**Simulation: 100-024**

Scenario: Scenario1  
Run Date/Time: 9/19/2022 12:52:18 PM  
Program Version: ICPRA 4.07.08

Run Mode: Normal

Year	Month	Day	Hour (hr)
0	0	0	0.0000
0	0	0	30.0000

Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
60.0000	0.1000	900.0000
Min Calculation Time:	60.0000	
Max Calculation Time:	60.0000	

**Output Time Increments**

Year	Month	Day	Hour (hr)	Time Increment (min)
0	0	0	0.0000	15.0000

**Resources & Linkup Tables**

Rainfall Folder: ICPR3  
Reference ET Folder:  
Unit Hydrograph Folder:  
Folder:

Boundary Stage Set:  
Extern Hydrograph Set:  
Curve Number Set: 1

Green-Ampt Set:  
Vertical Layers Set:  
Impervious Set: 1  
Roughness Set:  
Crop Coef Set:  
Fillable Porosity Set:

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Conductivity Set:  
Leakage Set:

### Tolerances & Options

Time Marching: SADR  
Max Iterations: 5  
Over-Relax Weight: 0.5 dec  
Fact:  
dZ Tolerance: 0.0010 ft  
Max dZ: 1.0000 ft  
Link Optimizer Tol: 0.0001 ft  
Edge Length Option: Automatic  
Dft Damping (2D): 0.0050 ft  
Min Node Srf Area: 1 ft<sup>2</sup>  
(2D):  
Energy Switch (2D): Energy  
IA Recovery Time: 24.0000 hr  
ET for Manual Basins: False  
Simp/Man Basin Rain Opt: Global  
OF Region Rain Opt: Global  
Rainfall Name: ~FDOT-24  
Rainfall Amount: 13.44 in  
Storm Duration: 24.0000 hr  
Dft Damping (1D): 0.0050 ft  
Min Node Srf Area: 113 ft<sup>2</sup>  
(1D):  
Energy Switch (1D): Energy

Comment:

### Simple Basin: Pre-Dev Basin

Scenario: Scenario1  
Node: Pre-Development Basin  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 31.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 256.0  
Area: 5.9210 ac  
Curve Number: 83.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-100

Scenario: Scenario1

Node: DS-100  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 99999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.6200 ac  
Curve Number: 76.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-101

Scenario: Scenario1  
Node: DS-101  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.4180 ac  
Curve Number: 75.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-102

Scenario: Scenario1  
Node: DS-102  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484

Peaking Factor: 484.0  
Area: 0.6390 ac  
Curve Number: 76.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-103

Scenario: Scenario1  
Node: DS-103  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.3720 ac  
Curve Number: 74.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-104

Scenario: Scenario1  
Node: DS-104  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.8070 ac  
Curve Number: 77.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-105

Scenario: Scenario1  
Node: DS-105  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.4740 ac  
Curve Number: 74.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-106

Scenario: Scenario1  
Node: DS-106  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.8000 ac  
Curve Number: 76.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:

### Simple Basin: SB-107

Scenario: Scenario1  
Node: DS-107  
Hydrograph Method: NRCS Unit Hydrograph  
Infiltration Method: Curve Number  
Time of Concentration: 10.0000 min  
Max Allowable Q: 9999.00 cfs  
Time Shift: 0.0000 hr  
Unit Hydrograph: UH484  
Peaking Factor: 484.0  
Area: 0.8000 ac  
Curve Number: 76.0  
% Impervious: 0.00  
% DCIA: 0.00  
% Direct: 0.00  
Rainfall Name:

Comment:



Scenario: Scenario1  
 Node: DS-108  
 Hydrograph Method: NRCS Unit Hydrograph  
 Infiltration Method: Curve Number  
 Time of Concentration: 10.0000 min  
 Max Allowable Q: 9999.00 cfs  
 Time Shift: 0.0000 hr  
 Unit Hydrograph: UH484  
 Peaking Factor: 484.0  
 Area: 0.5170 ac  
 Curve Number: 72.0  
 % Impervious: 0.00  
 % DCIA: 0.00  
 % Direct: 0.00  
 Rainfall Name:

Comment:

#### Simple Basin: SB-109

Scenario: Scenario1  
 Node: DS-109  
 Hydrograph Method: NRCS Unit Hydrograph  
 Infiltration Method: Curve Number  
 Time of Concentration: 10.0000 min  
 Max Allowable Q: 9999.00 cfs  
 Time Shift: 0.0000 hr  
 Unit Hydrograph: UH484  
 Peaking Factor: 484.0  
 Area: 0.1540 ac  
 Curve Number: 67.0  
 % Impervious: 0.00  
 % DCIA: 0.00  
 % Direct: 0.00  
 Rainfall Name:

Comment:

#### Simple Basin: SB-110

Scenario: Scenario1  
 Node: DS-110  
 Hydrograph Method: NRCS Unit Hydrograph  
 Infiltration Method: Curve Number  
 Time of Concentration: 10.0000 min  
 Max Allowable Q: 9999.00 cfs  
 Time Shift: 0.0000 hr

Unit Hydrograph: UH484  
 Peaking Factor: 484.0  
 Area: 0.1300 ac  
 Curve Number: 62.0  
 % Impervious: 0.00  
 % DCIA: 0.00  
 % Direct: 0.00  
 Rainfall Name:

Comment:

#### Simple Basin: SB-SWMP 1

Scenario: Scenario1  
 Node: SWMP 1  
 Hydrograph Method: NRCS Unit Hydrograph  
 Infiltration Method: Curve Number  
 Time of Concentration: 10.0000 min  
 Max Allowable Q: 99999.00 cfs  
 Time Shift: 0.0000 hr  
 Unit Hydrograph: UH484  
 Peaking Factor: 484.0  
 Area: 3.0600 ac  
 Curve Number: 90.0  
 % Impervious: 0.00  
 % DCIA: 0.00  
 % Direct: 0.00  
 Rainfall Name:

Comment:

#### Simple Basin: Unapportioned Area

Scenario: Scenario1  
 Node: Outfall  
 Hydrograph Method: NRCS Unit Hydrograph  
 Infiltration Method: Curve Number  
 Time of Concentration: 10.0000 min  
 Max Allowable Q: 99999.00 cfs  
 Time Shift: 0.0000 hr  
 Unit Hydrograph: UH484  
 Peaking Factor: 484.0  
 Area: 0.8800 ac  
 Curve Number: 39.0  
 % Impervious: 0.00  
 % DCIA: 0.00  
 % Direct: 0.00

Rainfall Name:

Comment:

#### Node: DS-100

Scenario: Scenario1  
 Type: Stage/Area  
 Base Flow: 0.00 cfs  
 Initial Stage: 7.00 ft  
 Warning Stage: 9.17 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
9.65	0.0006	25
9.70	0.0014	61
9.95	0.0002	9
9.17	0.0002	9

Comment: Rim Elev: 9.17  
 Sump Elev: 4.95  
 Desc: FDOT Type V Inlet

#### Node: DS-102

Scenario: Scenario1  
 Type: Stage/Area  
 Base Flow: 0.00 cfs  
 Initial Stage: 7.00 ft  
 Warning Stage: 9.17 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
9.64	0.0006	25
9.70	0.0011	50
5.41	0.0002	9
9.17	0.0002	9

Comment: Rim Elev: 9.46  
 Sump Elev: 4.91  
 Desc: FDOT Type V Inlet

#### Node: DS-107

Scenario: Scenario1  
 Type: Stage/Area  
 Base Flow: 0.00 cfs

Initial Stage: 7.00 ft  
 Warning Stage: 9.30 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
9.75	0.0011	50
9.80	0.0024	104
9.90	0.0050	217
10.00	0.0149	650
6.05	0.0002	9
9.30	0.0002	9

Comment: Rim Elev: 9.55  
 Sump Elev: 5.05  
 Desc: FDOT Type V Inlet

#### Node: DS-109

Scenario: Scenario1  
 Type: Stage/Area  
 Base Flow: 0.00 cfs  
 Initial Stage: 7.00 ft  
 Warning Stage: 9.30 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
9.78	0.0006	25
9.80	0.0014	63
9.90	0.0046	200
10.00	0.0138	600
10.10	0.0298	1300
5.10	0.0002	9
9.30	0.0002	9

Comment: Rim Elev: 9.30  
 Sump Elev: 4.10  
 Desc: FDOT Type V Inlet

#### Node: DS-107

Scenario: Scenario1  
 Type: Stage/Area  
 Base Flow: 0.00 cfs  
 Initial Stage: 7.00 ft  
 Warning Stage: 10.81 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
4.80	0.0002	9
10.81	0.0002	9



Comment: Rim Elev: 10.81  
Sump Elev: 3.80  
Desc: FDOT Type P7 Manhole

## Node: 05-105

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 9.91 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
6.10	0.0002	9
9.91	0.0002	9
10.12	0.0011	50
10.20	0.0025	108
10.30	0.0069	300
10.40	0.0241	1050

Comment: Rim Elev: 9.91  
Sump Elev: 5.10  
Desc: FDOT Type V Inlet

## Node: 05-106

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 9.91 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
10.11	0.0006	25
10.20	0.0017	75
10.30	0.0075	325
10.40	0.0218	950
9.96	0.0002	9
9.91	0.0002	9

Comment: Rim Elev: 9.91  
Sump Elev: 3.46  
Desc: FDOT Type V Inlet

## Node: 05-107

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 10.13 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
10.23	0.0006	25
10.25	0.0006	27
10.30	0.0008	34
10.35	0.0009	40
10.40	0.0011	46
10.45	0.0032	138
10.50	0.0063	275
3.40	0.0002	9
10.13	0.0002	9

Comment: Rim Elev: 10.13  
Sump Elev: 5.60  
Desc: FDOT Type V Inlet

## Node: 05-108

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 3.36 ft  
Warning Stage: 10.13 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
10.25	0.0006	25
10.30	0.0008	33
10.40	0.0011	48
10.50	0.0052	225
10.60	0.0207	900
2.86	0.0002	9
10.13	0.0002	9

Comment: Rim Elev: 10.13  
Sump Elev: 2.36  
Desc: FDOT Type V Inlet

## Node: 05-109

Scenario: Scenario1  
Type: Stage/Area

Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 10.48 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
10.60	0.0006	25
10.71	0.0011	46
10.80	0.0017	75
10.90	0.0075	325
6.95	0.0002	9
10.48	0.0002	9

Comment: Rim Elev: 10.48  
Sump Elev: 5.95  
Desc: FDOT Type V Inlet

## Node: 05-110

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 10.48 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
10.62	0.0006	25
10.70	0.0010	43
10.80	0.0023	100
10.90	0.0086	375
6.91	0.0002	9
10.48	0.0002	9

Comment: Rim Elev: 10.48  
Sump Elev: 5.91  
Desc: FDOT Type V Inlet

## Node: 05-111

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 11.37 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
2.47	0.0002	9
11.37	0.0002	9

Comment: Rim Elev: 11.37  
Sump Elev: 1.97  
Desc: FDOT Type P7 Manhole

## Node: 05-112

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 9.50 ft

Stage [ft]	Area [ac]	Area [ft <sup>2</sup> ]
2.26	0.0002	9
9.50	0.0002	9

Comment: Rim Elev: 9.50  
Sump Elev: 1.76  
Desc: FDOT Type P7 Manhole

## Node: 05-113

Scenario: Scenario1  
Type: Time/Stage  
Base Flow: 0.00 cfs  
Initial Stage: 6.75 ft  
Warning Stage: 7.00 ft  
Boundary Stage:

Area	Flow	Time	Node	Boundary
0	0	0	0.0000	6.75
0	0	0	9999999.0000	6.75

Comment:

## Node: 05-114 Development Basin

Scenario: Scenario1  
Type: Time/Stage  
Base Flow: 0.00 cfs  
Initial Stage: 0.00 ft  
Warning Stage: 1.00 ft  
Boundary Stage:

Year	Month	Day	Hour	Stage (ft)
0	0	0	0.0000	0.00
0	0	0	99999.0000	0.00

Comment:

Node: SWMP

Scenario: Scenario1  
Type: Stage/Area  
Base Flow: 0.00 cfs  
Initial Stage: 7.00 ft  
Warning Stage: 9.00 ft

Stage (ft)	Area (sq ft)	Area (T2)
-5.00	0.4192	18260
-4.00	0.4794	20663
-3.00	0.5420	23610
-2.00	0.6069	26437
-1.00	0.6739	29355
0.00	0.7431	32369
1.00	0.8145	35480
2.00	0.8882	38690
3.00	0.9638	41963
4.00	1.0089	43948
5.00	1.2033	52416
6.00	1.3712	59229
7.00	1.5415	67148
8.00	1.7140	74662
9.00	1.8888	82276

Comment:

Pipe Link: DP-101

Scenario	Scenario1	Scenario1
From Node	DS-100	DS-101
To Node	DS-101	DS-102
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

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Energy Switch: Energy	Ref Node: Manning's N: 0.0000	Ref Node: Manning's N: 0.0000
Comment: Material: HP STORM PP		

Pipe Link: DP-102

Scenario	Scenario1	Scenario1
From Node	DS-101	DS-102
To Node	DS-102	DS-103
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	154.20 ft	154.20 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.00	0.00
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

Pipe Link: DP-103

Scenario	Scenario1	Scenario1
From Node	DS-102	DS-103
To Node	DS-103	DS-104
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

Pipe Link: DP-104

Scenario	Scenario1	Scenario1
From Node	DS-103	DS-104
To Node	DS-104	DS-105
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

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Link Count: 1	Max Depth: 2.01 ft	Max Depth: 2.01 ft
Flow Direction: Both	Default: 0.00 ft	Default: 0.00 ft
Damping: 0.0000 ft	Op Table: Manning's N: 0.0000	Op Table: Manning's N: 0.0000
Length: 150.38 ft		
FHWA Code: 0		
Entr Loss Coef: 0.50	Default: 0.00 ft	Default: 0.00 ft
Exit Loss Coef: 0.50	Op Table: Manning's N: 0.0000	Op Table: Manning's N: 0.0000
Bend Loss Coef: 0.00		
Bend Location: 0.00 dec		
Energy Switch: Energy		

Comment: Material: HP STORM PP

Pipe Link: DP-105

Scenario	Scenario1	Scenario1
From Node	DS-104	DS-105
To Node	DS-105	DS-106
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	167.83 ft	167.83 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.00	0.00
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

Pipe Link: DP-106

Scenario	Scenario1	Scenario1
From Node	DS-105	DS-106
To Node	DS-106	DS-107
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

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Comment: Material: HP STORM PP

Pipe Link: DP-107

Scenario	Scenario1	Scenario1
From Node	DS-106	DS-107
To Node	DS-107	DS-108
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	298.73 ft	298.73 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.00	0.00
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

Pipe Link: DP-108

Scenario	Scenario1	Scenario1
From Node	DS-107	DS-108
To Node	DS-108	DS-109
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

Pipe Link: DP-109

Scenario	Scenario1	Scenario1
From Node	DS-108	DS-109
To Node	DS-109	DS-110
Link Count	1	1
Flow Direction	Both	Both
Damping	0.0000 ft	0.0000 ft
Length	20.00 ft	20.00 ft
FHWA Code	0	0
Entr Loss Coef	0.50	0.50
Exit Loss Coef	0.50	0.50
Bend Loss Coef	0.70	0.70
Bend Location	0.00 dec	0.00 dec
Energy Switch	Energy	Energy

Comment: Material: HP STORM PP

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Damping: 0.0000 ft  
Length: 192.73 ft  
FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 0.50  
Bend Loss Coef: 0.70  
Bend Location: 0.00 dec  
Energy Switch: Energy

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Comment: Material: HP STORM PP

Scenario: Scenario1  
From Node: DS-109  
To Node: DS-110  
Link Count: 1  
Flow Direction: Both  
Damping: 0.0000 ft  
Length: 25.00 ft  
FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 0.50  
Bend Loss Coef: 0.70  
Bend Location: 0.00 dec  
Energy Switch: Energy

Invert: 6.95 ft  
Manning's N: 0.0120  
Max Depth: 1.51 ft

Invert: 6.91 ft  
Manning's N: 0.0120  
Max Depth: 1.51 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Comment: Material: HP STORM PP

Scenario: Scenario1  
From Node: DS-110  
To Node: DS-111  
Link Count: 1  
Flow Direction: Both  
Damping: 0.0000 ft  
Length: 137.33 ft  
FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 0.50  
Bend Loss Coef: 0.00  
Bend Location: 0.00 dec  
Energy Switch: Energy

Invert: 6.91 ft  
Manning's N: 0.0120  
Max Depth: 1.51 ft

Invert: 3.69 ft  
Manning's N: 0.0120  
Max Depth: 1.51 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Comment: Material: HP STORM PP

Scenario: Scenario1  
From Node: DS-111  
To Node: DS-112  
Link Count: 1  
Flow Direction: Both  
Damping: 0.0000 ft  
Length: 102.59 ft  
FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 0.50  
Bend Loss Coef: 0.33  
Bend Location: 0.00 dec  
Energy Switch: Energy

Invert: 2.47 ft  
Manning's N: 0.0120  
Max Depth: 3.00 ft

Invert: 2.26 ft  
Manning's N: 0.0120  
Max Depth: 3.00 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Comment: Material: HP STORM PP

Scenario: Scenario1  
From Node: DS-112  
To Node: SWNF 1  
Link Count: 1  
Flow Direction: Both  
Damping: 0.0000 ft  
Length: 36.45 ft  
FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 1.00  
Bend Loss Coef: 0.00  
Bend Location: 0.00 dec  
Energy Switch: Energy

Invert: 2.25 ft  
Manning's N: 0.0120  
Max Depth: 3.00 ft

Invert: 1.50 ft  
Manning's N: 0.0120  
Max Depth: 3.00 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Comment: Material: HP STORM PP

Scenario: Scenario1  
From Node: SWNF 1  
To Node: Outfall  
Link Count: 1  
Flow Direction: Both  
Solutions: Combine  
Increments: 0  
Pipe Count: 1  
Damping: 0.0000 ft  
Length: 70.56 ft

Invert: 3.00 ft  
Manning's N: 0.0120  
Max Depth: 1.50 ft

Invert: 3.00 ft  
Manning's N: 0.0120  
Max Depth: 1.50 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

FHWA Code: 0  
Entr Loss Coef: 0.50  
Exit Loss Coef: 1.00  
Bend Loss Coef: 0.50  
Bend Location: 0.00 dec  
Energy Switch: Energy

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Pipe Comment:

Weir: 1  
Weir Count: 1  
Weir Flow Direction: Both  
Damping: 0.0000 ft  
Weir Type: Sharp Crested Vertical  
Geometry Type: Circular  
Invert: 5.50 ft  
Control Elevation: 7.00 ft  
Max Depth: 0.19 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Weir Default: 3.200  
Weir Table:  
Orifice Default: 0.600  
Orifice Table:

Weir Comment:

Weir: 2  
Weir Count: 2  
Weir Flow Direction: Both  
Damping: 0.0000 ft  
Weir Type: Sharp Crested Vertical  
Geometry Type: Rectangular  
Invert: 7.50 ft  
Control Elevation: 7.50 ft  
Max Depth: 1.08 ft  
Max Width: 1.50 ft  
Fillet: 0.00 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Weir Default: 3.200  
Weir Table:  
Orifice Default: 0.600  
Orifice Table:

Weir Comment:

Weir: 3  
Weir Count: 1  
Weir Flow Direction: Both  
Damping: 0.0000 ft  
Weir Type: Horizontal  
Geometry Type: Rectangular  
Invert: 8.75 ft  
Control Elevation: 8.75 ft  
Max Depth: 3.00 ft

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Default: 0.00 ft  
Op Table:  
Ref Node:  
Manning's N: 0.0000

Max Width: 2.00 ft  
Fillet: 0.00 ft

Weir Default: 3.200  
Weir Table:  
Orifice Default: 0.600  
Orifice Table:

Weir Comment:

Drop Structure Comment:

Node Max Conditions (Scenario1)

Node	Scenario	Manning's N	Entr Loss	Exit Loss	Bend Loss	Max Depth	Max Width	Max Velocity
DS-100	002-004 (SCS III)	9.17	8.78	-0.0011	1.96	1.91	100	
DS-100	025-001	9.17	8.94	-0.0010	2.48	2.34	113	
DS-100	025-002	9.17	8.91	-0.0010	2.33	2.22	113	
DS-100	025-004	9.17	8.39	-0.0010	1.94	1.31	113	
DS-100	025-008	9.17	8.82	-0.0010	1.64	1.60	113	
DS-100	025-024	9.17	8.19	-0.0010	0.58	1.17	113	
DS-100	100-001	9.17	10.11	-0.0010	3.28	3.09	113	
DS-100	100-002	9.17	10.05	-0.0010	3.08	2.92	113	
DS-100	100-004	9.17	8.98	-0.0010	1.81	1.77	113	
DS-100	100-008	9.17	8.63	-0.0010	2.13	2.09	113	
DS-100	100-024	9.17	8.44	-0.0010	0.75	1.17	113	

Node Max Conditions (Scenario1)

Node	Scenario	Manning's N	Entr Loss	Exit Loss	Bend Loss	Max Depth	Max Width	Max Velocity
DS-101	002-024 (SCS III)	9.17	8.75	-0.0012	3.18	3.15	170	
DS-101	025-001	9.17	8.89	-0.0012	3.82	3.84	169	
DS-101	025-002	9.17	8.86	-0.0012	3.73	3.65	169	
DS-101	025-004	9.17	8.38	-0.0012	2.19	2.95	169	
DS-101	025-008	9.17	8.80	-0.0012	2.69	2.95	169	
DS-101	025-024	9.17	8.19	-0.0012	1.32	2.95	169	
DS-101	100-001	9.17	10.07	-0.0012	5.17	5.08	169	
DS-101	100-002	9.17	9.97	-0.0012	4.91	4.82	169	
DS-101	100-004	9.17	8.95	-0.0012	2.96	2.95	169	
DS-101	100-008	9.17	8.60	-0.0012	3.50	3.46	169	
DS-101	100-024	9.17	8.44	-0.0012	1.32	2.95	169	



Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-102	002-024 (SCS III)	9.30	6.74	-0.0012	1.95	1.90	100
DS-102	025-001	9.30	8.88	-0.0011	2.48	2.33	113
DS-102	025-002	9.30	8.86	-0.0011	2.33	2.22	113
DS-102	025-004	9.30	8.37	-0.0011	1.34	1.30	113
DS-102	025-008	9.30	8.80	-0.0011	1.63	1.60	113
DS-102	025-024	9.30	8.19	-0.0011	0.58	1.24	113
DS-102	100-001	9.30	10.00	-0.0011	3.28	3.10	690
DS-102	100-002	9.30	9.96	-0.0011	3.08	2.92	469
DS-102	100-004	9.30	8.55	-0.0011	1.81	1.76	113
DS-102	100-008	9.30	9.58	-0.0011	2.12	2.08	113
DS-102	100-024	9.30	8.44	-0.0011	0.75	1.24	113

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-103	002-024 (SCS III)	9.30	8.70	-0.0014	6.17	7.13	282
DS-103	025-001	9.30	8.82	-0.0014	7.51	7.45	279
DS-103	025-002	9.30	8.81	-0.0014	7.17	7.11	279
DS-103	025-004	9.30	8.36	-0.0014	4.22	7.10	279
DS-103	025-008	9.30	8.77	-0.0014	5.21	7.10	279
DS-103	025-024	9.30	8.18	-0.0014	4.17	7.10	279
DS-103	100-001	9.30	9.92	-0.0014	9.73	9.59	279
DS-103	100-002	9.30	9.89	-0.0014	9.33	9.23	279
DS-103	100-004	9.30	8.92	-0.0014	5.72	7.10	279
DS-103	100-008	9.30	9.54	-0.0014	6.79	7.10	279
DS-103	100-024	9.30	8.43	-0.0014	4.17	7.10	279

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-104	002-024 (SCS III)	10.81	8.56	-0.0040	7.13	9.75	241
DS-104	025-001	10.81	8.61	-0.0036	7.45	9.79	237
DS-104	025-002	10.81	8.61	-0.0036	7.11	9.79	237
DS-104	025-004	10.81	8.33	-0.0036	7.10	9.79	237
DS-104	025-008	10.81	8.67	-0.0036	7.10	9.79	237
DS-104	025-024	10.81	8.18	-0.0036	7.10	9.79	237

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Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-107	002-024 (SCS III)	10.13	8.25	-0.0148	7.44	14.78	100
DS-107	025-001	10.13	8.15	-0.0131	7.44	14.78	113
DS-107	025-002	10.13	8.19	-0.0131	7.44	14.78	113
DS-107	025-004	10.13	8.26	-0.0131	7.44	14.78	113
DS-107	025-008	10.13	8.44	-0.0131	7.44	14.78	113
DS-107	025-024	10.13	8.16	-0.0131	7.44	14.78	113
DS-107	100-001	10.13	8.77	-0.0131	7.44	14.78	113
DS-107	100-002	10.13	8.82	-0.0131	7.44	14.78	113
DS-107	100-004	10.13	8.62	-0.0131	7.44	14.78	113
DS-107	100-008	10.13	8.98	-0.0131	7.44	14.78	113
DS-107	100-024	10.13	8.36	-0.0131	7.44	14.78	113

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-108	002-024 (SCS III)	10.13	8.19	0.0282	73.21	13.58	579
DS-108	025-001	10.13	8.07	0.0258	73.24	17.02	575
DS-108	025-002	10.13	8.12	0.0258	73.24	16.24	575
DS-108	025-004	10.13	8.25	0.0258	73.24	9.58	575
DS-108	025-008	10.13	8.40	0.0258	73.24	11.83	575
DS-108	025-024	10.13	8.16	0.0258	73.24	9.18	575
DS-108	100-001	10.13	8.63	0.0258	73.24	22.28	575
DS-108	100-002	10.13	8.69	0.0258	73.24	21.28	575
DS-108	100-004	10.13	8.60	0.0258	73.24	12.97	575
DS-108	100-008	10.13	8.91	0.0258	73.24	15.41	575
DS-108	100-024	10.13	8.35	0.0258	73.24	9.18	575

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-109	002-024 (SCS III)	10.48	8.04	0.0010	0.38	0.50	100
DS-109	025-001	10.48	7.85	0.0008	0.41	0.40	113
DS-109	025-002	10.48	8.02	0.0009	0.42	0.45	113
DS-109	025-004	10.48	8.22	0.0009	0.26	0.45	113
DS-109	025-008	10.48	8.33	0.0010	0.34	0.53	113
DS-109	025-024	10.48	8.15	0.0005	0.12	0.25	113

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Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-104	100-001	10.81	9.55	-0.0036	9.59	9.79	237
DS-104	100-002	10.81	9.54	-0.0036	9.23	9.79	237
DS-104	100-004	10.81	8.62	-0.0036	7.10	9.79	237
DS-104	100-008	10.81	9.36	-0.0036	7.10	9.79	237
DS-104	100-024	10.81	8.41	-0.0036	7.10	9.79	237

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-105	002-024 (SCS III)	9.91	8.46	-0.0022	2.60	2.56	100
DS-105	025-001	9.91	8.47	-0.0020	3.36	3.23	113
DS-105	025-002	9.91	8.49	-0.0020	3.13	3.03	113
DS-105	025-004	9.91	8.31	-0.0020	1.79	2.23	113
DS-105	025-008	9.91	8.60	-0.0020	2.16	2.23	113
DS-105	025-024	9.91	8.17	-0.0020	1.01	2.23	113
DS-105	100-001	9.91	9.32	-0.0020	4.41	4.21	113
DS-105	100-002	9.91	9.33	-0.0020	4.12	3.96	113
DS-105	100-004	9.91	8.75	-0.0020	2.41	2.37	113
DS-105	100-008	9.91	9.24	-0.0020	2.80	2.76	113
DS-105	100-024	9.91	8.39	-0.0020	1.01	2.23	113

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-106	002-024 (SCS III)	9.91	8.40	-0.0020	12.29	23.68	402
DS-106	025-001	9.91	8.38	-0.0195	12.27	23.68	395
DS-106	025-002	9.91	8.40	-0.0195	11.82	23.68	395
DS-106	025-004	9.91	8.30	-0.0195	11.82	23.68	395
DS-106	025-008	9.91	8.55	-0.0195	11.82	23.68	395
DS-106	025-024	9.91	8.17	-0.0195	11.82	23.68	395
DS-106	100-001	9.91	9.16	-0.0195	15.58	23.68	395
DS-106	100-002	9.91	9.18	-0.0195	15.27	23.68	395
DS-106	100-004	9.91	8.70	-0.0195	11.82	23.68	395
DS-106	100-008	9.91	9.17	-0.0195	11.82	23.68	395
DS-106	100-024	9.91	8.38	-0.0195	11.82	23.68	395

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Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-109	100-001	10.48	8.24	0.0009	0.58	0.66	113
DS-109	100-002	10.48	8.32	0.0010	0.59	0.69	113
DS-109	100-004	10.48	8.54	0.0010	0.36	0.55	113
DS-109	100-008	10.48	8.72	0.0010	0.46	0.64	113
DS-109	100-024	10.48	8.33	-0.0007	0.17	0.39	113

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-110	002-024 (SCS III)	10.48	8.03	-0.0010	0.76	0.68	100
DS-110	025-001	10.48	7.85	-0.0009	0.66	0.62	113
DS-110	025-002	10.48	8.02	-0.0010	0.72	0.65	113
DS-110	025-004	10.48	8.22	0.0010	0.63	0.61	113
DS-110	025-008	10.48	8.32	0.0010	0.78	0.65	113
DS-110	025-024	10.48	8.15	-0.0008	0.34	0.46	113
DS-110	100-001	10.48	8.24	0.0010	0.98	0.88	113
DS-110	100-002	10.48	8.32	-0.0010	1.08	0.92	113
DS-110	100-004	10.48	8.54	-0.0010	0.81	0.66	113
DS-110	100-008	10.48	8.72	0.0010	0.98	0.79	113
DS-110	100-024	10.48	8.33	-0.0010	0.52	0.57	113

Node Max Conditions (Scenario1)							
Node Number	Node Name	Water Depth (ft)	Water Level (ft)	Water Velocity (ft/s)	Water Temperature (°F)	Water Density (lbm/ft³)	Water Viscosity (cP)
DS-111	002-024 (SCS III)	11.37	8.63	-0.0296	27.29	34.76	100
DS-111	025-001	11.37	7.94	-0.0265	27.16	34.78	113
DS-111	025-002	11.37	8.02	-0.0265	27.16	34.78	113
DS-111	025-004	11.37	8.22	-0.0265	27.16	34.78	113
DS-111	025-008	11.37	8.32	-0.0265	27.16	34.78	113
DS-111	025-024	11.37	8.15	-0.0265	27.16	34.78	113
DS-111	100-001	11.37	8.23	-0.0265	27.16	34.78	113
DS-111	100-002	11.37	8.31	-0.0265	27.16	34.78	113
DS-111	100-004	11.37	8.54	-0.0265	27.16	34.78	113
DS-111	100-008	11.37	8.71	-0.0265	27.16	34.78	113
DS-111	100-024	11.37	8.33	-0.0265	27.16	34.78	113

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Node Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DS-112	002-024 (SCS III)	9.50	7.98	-0.0065	26.87	27.25	100
DS-112	025-001	9.50	7.76	-0.0056	26.64	27.12	113
DS-112	025-002	9.50	8.01	-0.0056	26.64	27.12	113
DS-112	025-004	9.50	8.20	-0.0056	26.64	27.12	113
DS-112	025-008	9.50	8.31	-0.0056	26.64	27.12	113
DS-112	025-024	9.50	8.14	-0.0056	26.64	27.12	113
DS-112	100-001	9.50	7.99	-0.0056	26.64	27.12	113
DS-112	100-002	9.50	8.23	-0.0056	26.64	27.12	113
DS-112	100-004	9.50	8.49	-0.0056	26.64	27.12	113
DS-112	100-008	9.50	8.61	-0.0056	26.64	27.12	113
DS-112	100-024	9.50	8.31	-0.0056	26.64	27.12	113

Node Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
Outfall	002-024 (SCS III)	7.00	6.75	0.0000	3.38	0.00	0
Outfall	025-001	7.00	6.75	0.0000	1.41	0.00	0
Outfall	025-002	7.00	6.75	0.0000	3.72	0.00	0
Outfall	025-004	7.00	6.75	0.0000	5.78	0.00	0
Outfall	025-008	7.00	6.75	0.0000	7.21	0.00	0
Outfall	025-024	7.00	6.75	0.0000	5.23	0.00	0
Outfall	100-001	7.00	6.75	0.0000	6.18	0.00	0
Outfall	100-002	7.00	6.75	0.0000	8.86	0.00	0
Outfall	100-004	7.00	6.75	0.0000	9.75	0.00	0
Outfall	100-008	7.00	6.75	0.0000	7.34	0.00	0
Outfall	100-024	7.00	6.75	0.0000	7.34	0.00	0

Node Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
Pre-Development ent Basin	002-024 (SCS III)	1.00	0.00	0.0000	11.21	0.00	0
Pre-Development ent Basin	025-001	1.00	0.00	0.0000	11.87	0.00	0
Pre-Development ent Basin	025-002	1.00	0.00	0.0000	12.27	0.00	0
Pre-Development ent Basin	025-004	1.00	0.00	0.0000	11.05	0.00	0

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
ent Basin	025-008	1.00	0.00	0.0000	12.86	0.00	0
Pre-Development ent Basin	025-024	1.00	0.00	0.0000	5.45	0.00	0
Pre-Development ent Basin	100-001	1.00	0.00	0.0000	15.23	0.00	0
Pre-Development ent Basin	100-002	1.00	0.00	0.0000	15.88	0.00	0
Pre-Development ent Basin	100-004	1.00	0.00	0.0000	14.43	0.00	0
Pre-Development ent Basin	100-008	1.00	0.00	0.0000	16.60	0.00	0
Pre-Development ent Basin	100-024	1.00	0.00	0.0000	6.58	0.00	0

Node Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
SWMF 1	002-024 (SCS III)	9.00	7.98	0.0006	26.75	26.87	74494
SWMF 1	025-001	9.00	7.76	0.0009	35.92	26.64	72867
SWMF 1	025-002	9.00	8.01	0.0009	32.65	26.64	74736
SWMF 1	025-004	9.00	8.19	0.0005	18.68	26.64	76980
SWMF 1	025-008	9.00	8.29	0.0004	21.78	26.64	76902
SWMF 1	025-024	9.00	8.14	0.0001	7.92	26.64	75716
SWMF 1	100-001	9.00	7.95	0.0010	45.59	26.64	74347
SWMF 1	100-002	9.00	8.22	0.0008	41.58	26.64	76329
SWMF 1	100-004	9.00	8.47	0.0005	24.58	26.64	76123
SWMF 1	100-008	9.00	8.59	0.0005	17.61	26.64	79181
SWMF 1	100-024	9.00	8.10	0.0001	9.97	26.64	76932

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-100	002-024 (SCS III)	1.91	-0.54	-0.51	1.07	1.07	1.07
DP-100	025-001	2.34	-0.56	-0.49	1.31	1.31	1.31
DP-100	025-002	2.22	-0.56	0.53	1.25	1.25	1.25
DP-100	025-004	1.31	-0.56	0.52	1.13	1.13	1.13

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-100	025-008	1.60	-0.56	-0.50	1.13	1.13	1.13
DP-100	025-024	1.17	-0.56	0.58	1.13	1.13	1.13
DP-100	100-001	3.09	-0.56	-0.50	1.74	1.74	1.74
DP-100	100-002	2.92	-0.56	0.56	1.64	1.64	1.64
DP-100	100-004	1.77	-0.56	0.56	1.13	1.13	1.13
DP-100	100-008	2.09	-0.56	-0.50	1.17	1.17	1.17
DP-100	100-024	1.17	-0.56	0.58	1.13	1.13	1.13

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-101	002-024 (SCS III)	3.15	-1.38	0.96	1.27	1.04	1.16
DP-101	025-001	3.84	-1.32	-0.90	1.28	1.22	1.22
DP-101	025-002	3.66	-1.32	0.92	1.28	1.16	1.17
DP-101	025-004	2.95	-1.32	-0.98	1.28	1.06	1.17
DP-101	025-008	2.95	-1.32	-0.95	1.28	1.05	1.17
DP-101	025-024	2.95	-1.32	-1.00	1.28	1.06	1.17
DP-101	100-001	5.08	-1.32	0.92	1.62	1.62	1.62
DP-101	100-002	4.82	-1.32	0.88	1.53	1.53	1.53
DP-101	100-004	2.95	-1.32	0.94	1.28	1.06	1.17
DP-101	100-008	3.46	-1.32	-0.99	1.28	1.10	1.17
DP-101	100-024	2.95	-1.32	-1.02	1.28	1.06	1.17

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-102	002-024 (SCS III)	1.90	-0.54	0.55	1.25	1.28	1.27
DP-102	025-001	2.33	-0.56	0.49	1.33	1.38	1.35
DP-102	025-002	2.22	-0.56	0.49	1.33	1.38	1.35
DP-102	025-004	1.30	-0.56	0.54	1.33	1.38	1.35
DP-102	025-008	1.60	-0.56	0.50	1.33	1.38	1.35
DP-102	025-024	1.24	-0.56	-0.63	1.33	1.38	1.35
DP-102	100-001	3.10	-0.56	0.50	1.74	1.74	1.74
DP-102	100-002	2.92	-0.56	0.48	1.64	1.64	1.64
DP-102	100-004	1.76	-0.56	0.56	1.33	1.38	1.35
DP-102	100-008	2.08	-0.56	0.61	1.33	1.38	1.35
DP-102	100-024	1.24	-0.56	0.62	1.33	1.38	1.35

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-103	002-024 (SCS III)	7.13	-3.22	0.90	2.50	2.36	2.43
DP-103	025-001	7.45	-3.03	-0.79	2.51	2.38	2.44
DP-103	025-002	7.11	-3.03	-0.95	2.51	2.38	2.44
DP-103	025-004	7.10	-3.03	-0.92	2.51	2.38	2.44
DP-103	025-008	7.10	-3.03	0.89	2.51	2.38	2.44
DP-103	025-024	7.10	-3.03	0.98	2.51	2.38	2.44
DP-103	100-001	9.59	-3.03	0.61	3.01	3.01	3.01
DP-103	100-002	9.23	-3.03	0.86	2.90	2.90	2.90
DP-103	100-004	7.10	-3.03	0.85	2.51	2.38	2.44
DP-103	100-008	7.10	-3.03	0.90	2.51	2.38	2.44
DP-103	100-024	7.10	-3.03	0.95	2.51	2.38	2.44

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-104	002-024 (SCS III)	9.75	-3.75	1.11	3.29	3.46	3.37
DP-104	025-001	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	025-002	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	025-004	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	025-008	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	025-024	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	100-001	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	100-002	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	100-004	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	100-008	9.79	-3.60	1.03	3.28	3.44	3.36
DP-104	100-024	9.79	-3.60	1.03	3.28	3.44	3.36

Link Min/Max Conditions [Scenario1]

Link Name	Scenario	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)	Min Flow (G)
DP-105	002-024 (SCS III)	2.56	-1.05	0.45	2.11	3.61	2.82
DP-105	025-001	3.23	-1.01	0.43	2.11	3.61	2.82
DP-105	025-002	3.03	-1.01	0.51	2.11	3.61	2.82
DP-105	025-004	2.23	-1.01	0.52	2.11	3.61	2.82
DP-105	025-008	2.23	-1.01	-0.53	2.11	3.61	2.82
DP-105	025-024	2.23	-1.01	-0.50	2.11	3.61	2.82



Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-105	100-001	4.21	-1.01	0.43	2.37	3.61	2.82
DP-105	100-002	3.96	-1.01	0.43	2.23	3.61	2.82
DP-105	100-004	2.37	-1.01	-0.51	2.11	3.61	2.82
DP-105	100-008	2.76	-1.01	0.48	2.11	3.61	2.82
DP-105	100-024	2.23	-1.01	0.56	2.11	3.61	2.82

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-106	002-024 (SCS III)	23.68	-5.44	1.39	4.82	6.88	5.85
DP-106	025-001	23.68	-5.26	-1.25	4.82	6.88	5.85
DP-106	025-002	23.68	-5.26	-1.25	4.82	6.88	5.85
DP-106	025-004	23.68	-5.26	1.36	4.82	6.88	5.85
DP-106	025-008	23.68	-5.26	1.31	4.82	6.88	5.85
DP-106	025-004	23.68	-5.26	1.30	4.82	6.88	5.85
DP-106	100-001	23.68	-5.26	1.26	4.82	6.88	5.85
DP-106	100-002	23.68	-5.26	-1.25	4.82	6.88	5.85
DP-106	100-004	23.68	-5.26	-1.25	4.82	6.88	5.85
DP-106	100-008	23.68	-5.26	-1.25	4.82	6.88	5.85
DP-106	100-024	23.68	-5.26	1.42	4.82	6.88	5.85

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-107	002-024 (SCS III)	14.78	-7.44	-1.13	8.31	8.56	8.43
DP-107	025-001	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	025-002	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	025-004	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	025-008	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	025-004	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	100-001	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	100-002	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	100-004	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	100-008	14.78	-7.44	-0.97	8.31	8.56	8.43
DP-107	100-024	14.78	-7.44	-0.97	8.31	8.56	8.43

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-108	002-024 (SCS III)	13.98	-34.76	2.89	-4.52	-7.30	-6.11
DP-108	025-001	17.02	-34.78	1.73	-4.52	-7.30	-6.11
DP-108	025-002	16.24	-34.78	1.84	-4.52	-7.30	-6.11
DP-108	025-004	9.58	-34.78	1.76	-4.52	-7.30	-6.11
DP-108	025-008	11.83	-34.78	2.15	-4.52	-7.30	-6.11
DP-108	025-024	4.50	-34.78	2.18	-4.52	-7.30	-6.11
DP-108	100-001	22.28	-34.78	1.82	-4.52	-7.30	-6.11
DP-108	100-002	21.28	-34.78	1.76	-4.52	-7.30	-6.11
DP-108	100-004	22.97	-34.78	1.85	-4.52	-7.30	-6.11
DP-108	100-008	15.41	-34.78	2.18	-4.52	-7.30	-6.11
DP-108	100-024	5.72	-34.78	2.12	-4.52	-7.30	-6.11

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-109	002-024 (SCS III)	0.50	-0.03	-0.32	0.37	0.35	0.36
DP-109	025-001	0.40	-0.01	-0.21	0.41	0.37	0.39
DP-109	025-002	0.45	-0.01	-0.33	0.43	0.37	0.40
DP-109	025-004	0.45	-0.01	-0.42	0.35	0.33	0.34
DP-109	025-008	0.53	0.00	-0.40	0.35	0.33	0.34
DP-109	025-024	0.25	0.00	-0.24	0.17	0.39	0.17
DP-109	100-001	0.60	-0.01	0.21	0.39	0.17	0.37
DP-109	100-002	0.69	-0.01	-0.43	0.43	0.40	0.40
DP-109	100-004	0.55	-0.01	-0.48	0.36	0.34	0.35
DP-109	100-008	0.64	0.00	-0.42	0.36	0.36	0.36
DP-109	100-024	0.39	0.00	-0.36	0.23	0.22	0.23

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-110	002-024 (SCS III)	0.68	-0.03	0.52	0.48	0.38	0.43
DP-110	025-001	0.62	-0.05	0.42	0.54	-0.60	0.44
DP-110	025-002	0.65	-0.04	0.52	0.52	-0.51	0.44
DP-110	025-004	0.61	-0.02	0.54	0.57	-0.35	0.39
DP-110	025-008	0.65	-0.02	0.55	0.55	0.37	0.39
DP-110	025-024	0.46	-0.01	-0.43	0.31	0.26	0.28

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-110	100-001	0.66	-0.05	-0.32	0.54	-0.61	0.52
DP-110	100-002	0.92	-0.04	0.54	0.53	-0.54	0.52
DP-110	100-004	0.66	-0.03	0.50	0.53	0.37	0.40
DP-110	100-008	0.79	-0.02	0.57	0.46	0.45	0.45
DP-110	100-024	0.57	-0.01	0.56	0.34	0.32	0.33

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-111	002-024 (SCS III)	14.53	-27.25	-4.59	-3.85	-3.85	-3.85
DP-111	025-001	17.52	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	025-002	16.78	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	025-004	9.95	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	025-008	12.35	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	025-024	5.64	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	100-001	23.02	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	100-002	22.07	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	100-004	13.48	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	100-008	16.13	-27.12	-4.35	-3.84	-3.84	-3.84
DP-111	100-024	6.10	-27.12	-4.35	-3.84	-3.84	-3.84

Link Min/Max Conditions [Scenario1]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
DP-112	002-024 (SCS III)	14.49	-26.87	-1.92	-3.80	-3.80	-3.80
DP-112	025-001	17.47	-26.64	-1.85	-3.77	-3.77	-3.77
DP-112	025-002	16.73	-26.64	-1.88	-3.77	-3.77	-3.77
DP-112	025-004	9.85	-26.64	-1.81	-3.77	-3.77	-3.77
DP-112	025-008	12.30	-26.64	-1.89	-3.77	-3.77	-3.77
DP-112	025-024	4.68	-26.64	-1.91	-3.77	-3.77	-3.77
DP-112	100-001	22.95	-26.64	-1.83	-3.77	-3.77	-3.77
DP-112	100-002	22.01	-26.64	-1.83	-3.77	-3.77	-3.77
DP-112	100-004	13.44	-26.64	-1.83	-3.77	-3.77	-3.77
DP-112	100-008	16.10	-26.64	-1.90	-3.77	-3.77	-3.77
DP-112	100-024	5.90	-26.64	-1.93	-3.77	-3.77	-3.77

Link Min/Max Conditions [Scenario1]

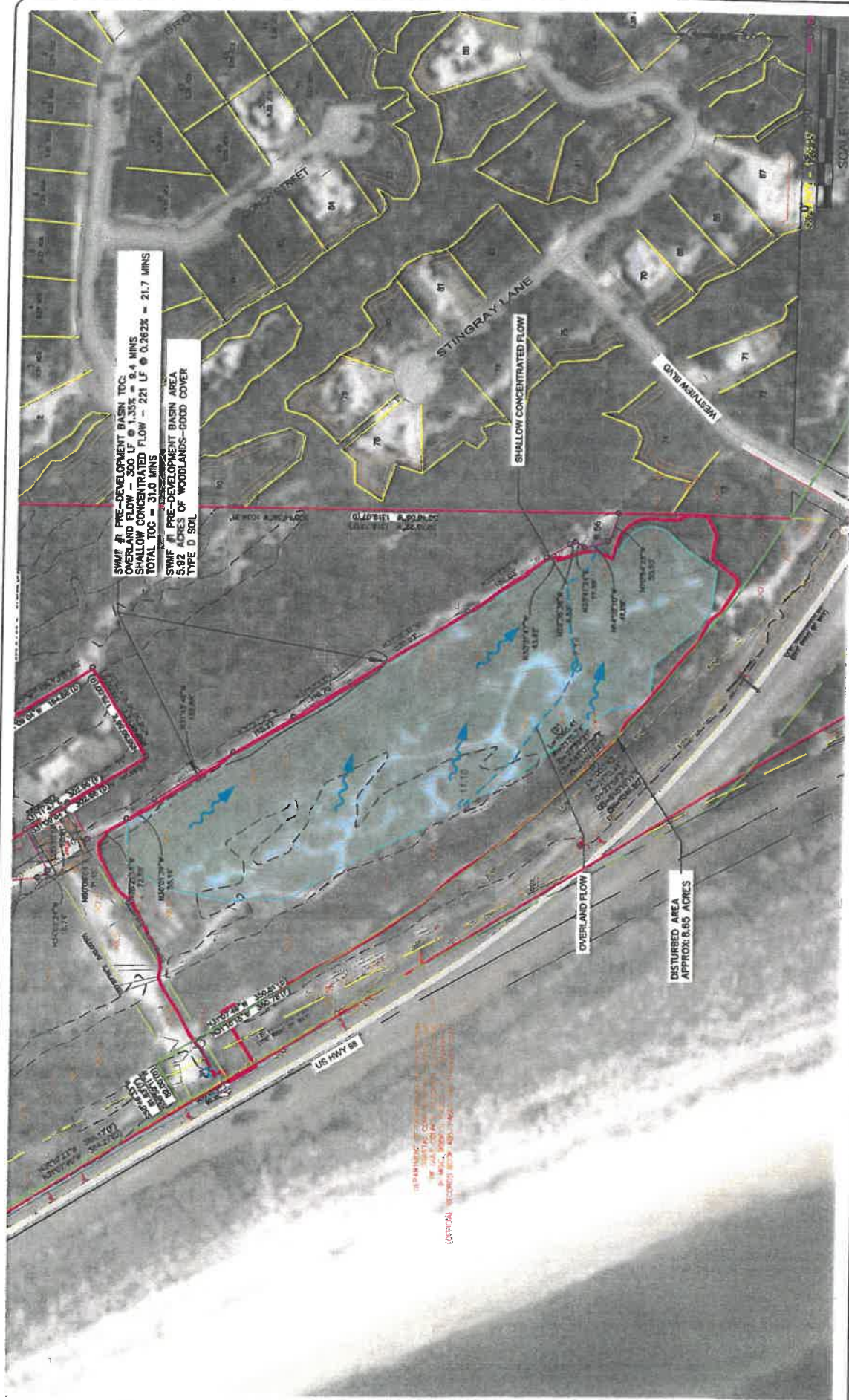
Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
SWMF Outfall - Pipe	002-024 (SCS III)	3.30	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	002-024 (SCS III)	0.13	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	002-024 (SCS III)	3.17	0.00	0.00	2.21	2.21	2.21
SWMF Outfall - Weir: 3	002-024 (SCS III)	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	025-001	1.40	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 1	025-001	0.12	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	025-001	1.28	0.00	0.00	1.63	1.63	1.63
SWMF Outfall - Weir: 3	025-001	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	025-002	3.62	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	025-002	0.13	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	025-002	3.49	0.00	0.00	2.28	2.28	2.28
SWMF Outfall - Weir: 3	025-002	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	025-004	5.58	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 1	025-004	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	025-004	5.46	0.00	0.00	2.65	2.65	2.65
SWMF Outfall - Weir: 3	025-004	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	025-008	6.91	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 1	025-008	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	025-008	6.79	0.00	-0.01	2.85	2.85	2.85
SWMF Outfall - Weir: 3	025-008	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	025-024	5.03	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 1	025-024	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	025-024	4.90	0.00	0.00	2.56	2.56	2.56

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Link Name	Sub-Link	Max Flow [cfs]	Min Flow [cfs]	Max/Min Flow Ratio	Max Dis Velocity [fps]	Max Dis Velocity [fps]	Max Dis Velocity [fps]
SWMF Outfall - Weir: 3	025-024	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	100-001	3.10	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	100-001	0.13	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	100-001	2.98	0.00	0.00	2.17	2.17	2.17
SWMF Outfall - Weir: 3	100-001	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	100-002	5.97	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	100-002	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	100-002	5.85	0.00	0.01	2.71	2.71	2.71
SWMF Outfall - Weir: 3	100-002	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	100-004	8.35	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	100-004	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	100-004	8.28	0.00	-0.01	2.89	2.89	2.89
SWMF Outfall - Weir: 3	100-004	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	100-008	9.24	0.00	0.01	0.00	0.00	0.00
SWMF Outfall - Weir: 1	100-008	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	100-008	9.14	0.00	0.00	2.89	2.89	2.89
SWMF Outfall - Weir: 3	100-008	0.00	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Pipe	100-024	6.99	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 1	100-024	0.14	0.00	0.00	0.00	0.00	0.00
SWMF Outfall - Weir: 2	100-024	6.87	0.00	0.00	2.86	2.86	2.86
SWMF Outfall - Weir: 3	100-024	0.00	0.00	0.00	0.00	0.00	0.00

## EXHIBIT 5 DRAINAGE BASIN MAPS





SWMF #1 PRE-DEVELOPMENT BASIN TOC  
 OVERLAND FLOW - 1.35% = 8.4 MINS  
 SHALLOW CONCENTRATED FLOW - 221 LF @ 0.262% = 21.7 MINS  
 TOTAL TOC = 31.0 MINS  
 SWMF #1 PRE-DEVELOPMENT BASIN AREA  
 5.82 ACRES OF WOODLANDS-GOOD COVER  
 TYPE D SOIL

DISTURBED AREA  
 APPROX: 8.65 ACRES

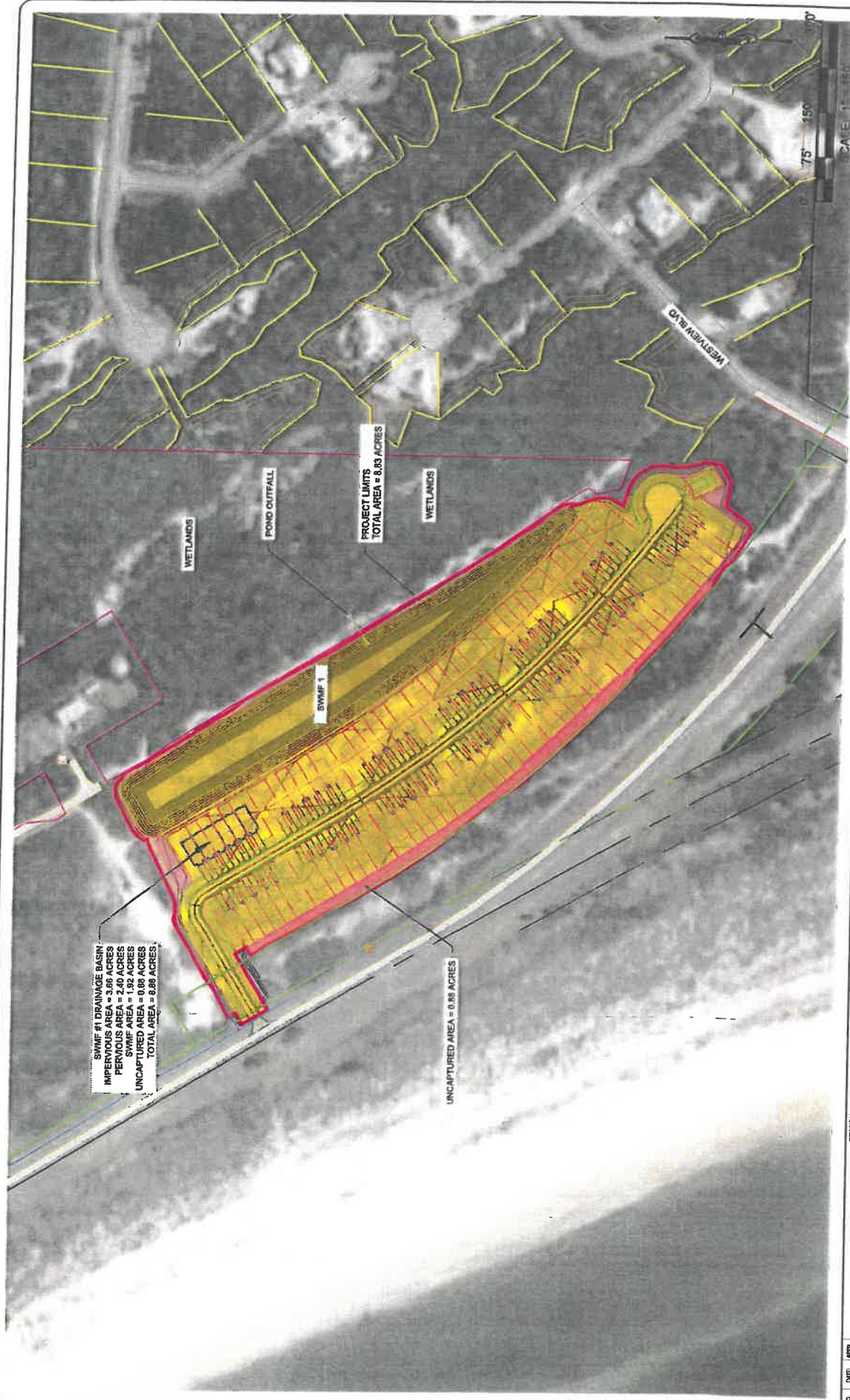
SHALLOW CONCENTRATED FLOW

OVERLAND FLOW

NO.	DATE	APPR.	REVISION	WINDMARK BEACH NORTH TOWNHOMES PRE-DEVELOPMENT DRAINAGE BASIN MAP		ESS 000794
	1			DATE	PROJECT NO.	
	2			DESIGNED BY	50144759	
	3			SCALE	1" = 150'	
4				DRAWN	DATE	APPROVED
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**Dewberry**  
 203 Aberdeen Parkway, Panama City, FL 32405  
 850.522.0644, www.dewberry.com





NO.		DATE	APP.
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5			
REVISION:			
<div>  <b>Dewberry</b>                      203 Aberdeen Parkway, Panama City, FL 32405                      850.522.0844, www.dewberry.com                 </div>			
WINDMARK BEACH NORTH TOWNHOMES POST-DEVELOPMENT DRAINAGE BASIN MAP			
DATE: 09/19/2022 PROJECT NO.: 50144759 SCALE: 1" = 150' DRAWN: JMB APPROVED: JMB		SHEET: 1 OF 2	



## EXHIBIT 6

# GEOTECHNICAL REPORT



**MAGNUM ENGINEERING INC**  
**GEOTECHNICAL ENGINEERING**  
**CONSULTANTS**

**GEOTECHNICAL ENGINEERING REPORT**

**WINDMARK NORTH TOWNHOMES  
GULF COUNTY, FLORIDA**

**PREPARED FOR:**

**Mr. Jonathan Sklarski, P.E.  
Dewberry Engineers, Inc.  
203 Aberdeen Parkway  
Panama City, Florida 32405**

**429 FLORIDA AVENUE  
LYNN HAVEN, FLORIDA 32444  
TELEPHONE (850) 258.0994**



**MAGNUM ENGINEERING INC**  
**GEOTECHNICAL ENGINEERING**  
**CONSULTANTS**

November 18, 2021

Mr. Jonathan Sklarski, P.E.  
Dewberry Engineers, Inc.  
203 Aberdeen Parkway  
Panama City, Florida 32405

**SUBJECT:** Windmark North Townhomes – Geotechnical Services for Roadway and Stormwater Ponds  
Gulf County, Florida  
MEI Project No. M121-100-330

Dear Mr. Sklarski:

This letter forwards the results of the geotechnical services performed for the proposed Windmark North Townhomes development in Gulf County, Florida. The purpose of this exploration was to determine soil types, groundwater depths, and the estimated seasonal high groundwater levels in the proposed roadway and stormwater pond areas. In addition, site/soil preparation recommendations and pavement recommendations have been provided for the proposed roadways.

**Project Description and Scope of Work**

The subject site is located east of US Highway 98, south of Pine Street and just north of the Sunset Village Subdivision in Gulf County, Florida. At the time of our exploration, the site was undeveloped, wooded with coastal dune vegetation, and had several trails traversing the property.

Our exploration consisted of performing a total of Ten (10) hand auger borings to a depth of 5-feet deep at the locations shown on the attached boring location plan identified as Figure #1.

**Subsurface Conditions**

The borings generally encountered light gray clean fine sands from the ground surface to the boring termination depths of 5-feet below existing grade. Surficial organics were encountered in borings HA-7 through HA-10. These areas consisted of wetland material and groundwater was encountered at the surface of the borings.

The above subsurface descriptions are of a generalized nature, provided to highlight the major soil strata encountered. The Logs of Boring should be reviewed for specific subsurface conditions at each boring location. The stratifications shown on the Logs of Boring represent the subsurface conditions at the actual boring locations only, and variations in the subsurface conditions can and may occur between boring locations and should therefore be expected. The stratifications represent the approximate boundary between subsurface materials, and the transitions between strata may be gradual.

Figure #1 shows the boring location plan and Figure #2 shows the Logs of Borings for borings HA-1 to HA-10.

### Groundwater Conditions

Groundwater was encountered between the surface and 2.8 feet below existing grade at the time of drilling (November 3, 2021), which was during a period of slightly above normal seasonal rainfall. By definition, the normal seasonal high groundwater table elevation is the highest level of the saturated zone in the soil during a year with normal rainfall. The procedure used in estimating the seasonal high groundwater table is based on adjusting the existing groundwater table encountered upward or downward and taking into consideration factors such as antecedent rainfall, redoximorphic features (identifying soil mottling) and vegetative indicators. The following Table #1 provides the groundwater levels and estimated seasonal high groundwater levels at each boring location. Groundwater levels will fluctuate with rainfall and could vary several feet during typical seasonal fluctuations. Larger fluctuations are possible under severe weather conditions.

**TABLE #1**  
**GROUNDWATER DATA**

TEST LOCATION	DEPTH TO EXISTING GROUNDWATER TABLE (ft)	DEPTH TO ESTIMATED SEASONAL HIGH GROUNDWATER TABLE (ft)
HA-1	1.5 feet	1.5 feet
HA-2	2.2 feet	2.2 feet
HA-3	2.7 feet	2.7 feet
HA-4	2.8 feet	2.8 feet
HA-5	2.2 feet	2.2 feet
HA-6	Surface	Surface
HA-7	Surface	Surface
HA-8	Surface	Surface
HA-9	Surface	Surface
HA-10	Surface	Surface

The following geotechnical related design recommendations have been developed on the basis of the previously described project characteristics and subsurface conditions encountered. If there are any changes in these project criteria, including project location on the site, a review should be made by Magnum Engineering to determine if modifications to the recommendations are warranted.

Once final design plans and specifications are available, a general review by Magnum Engineering is recommended as a means to check that the evaluations made in preparation of this report are correct and that earthwork and foundation recommendations are properly interpreted and implemented

### Site Preparation

The site should be cleared and grubbed of surface vegetation and any other deleterious material. As a minimum, it is recommended the clearing operations extend at least five feet beyond the development perimeters.

Fill and backfill, if required to raise site to final grades, should consist of sandy soils with less than 15 percent passing the No. 200 sieve. These soils should be free of rubble, organics, clay, debris and other unsuitable material. Fill should be placed in lifts on the order of 12 inches or less (in loose thickness) and compacted to 95 percent of the soil's Modified Proctor maximum dry density, per ASTM D-1557.

**Windmark North Townhomes - Geotechnical Services**  
**Gulf County, Florida**  
**Page 3 of 4**

Prior to placing fill soils, where applicable, the top of the ground surface should be compacted to a minimum soil density of 95% of the Modified Proctor Test (ASTM D1557).

Structural fill soils should be placed in maximum 12-inch lifts and compacted to a minimum soil density of 95% of the Modified Proctor Test (ASTM D1557). The top 12 inches of subgrade should be compacted to a minimum soil density of 98% of the Modified Proctor Test (ASTM D1557). The top 12 inches of subgrade should have a minimum LBR value of 40. We recommend that structural fill soils, where planned, have a minimum LBR of 40.

**Pavement Recommendations**

Based on the subsurface conditions encountered in the test borings, we recommend using a graded aggregate base (i.e. limerock or crushed concrete). The base course should be compacted to a minimum soil density of 98% of the Modified Proctor Test (ASTM D1557).

Without benefit of traffic loads, volumes, and serviceability parameters, a pavement section cannot be designed. However, typical pavement in the local area generally consist of a minimum of 1½ inches of FDOT Superpave Mix SP-12.5 or SP-9.5 asphaltic concrete and a minimum of 6 inches of base. Moderate duty traffic areas (e.g. main entrance areas) typically have a minimum pavement section consisting of 2 inches of FDOT Superpave Mix SP-12.5 asphaltic concrete and 8 inches of base.

The above sections represent minimum thicknesses representative of typical, local construction practices, and as such periodic maintenance should be anticipated. All pavement materials and construction procedures should conform to FDOT and/or appropriate city or county requirements

While specific traffic loads and volumes for the project have not been provided, we are providing recommended light-duty and medium-duty pavement sections, which have been successfully utilized for this type of commercial development in the Northwest Florida area.

**Light Duty (General roadway areas)**

- 1 ½ inches Asphalt Concrete (FDOT Superpave Mix SP-12.5 or SP-9.5)
- 6 inches Crushed Limerock or Graded Aggregate Base (minimum LBR 100)
- 12 inches stabilized subgrade (minimum LBR 40)

**Medium Duty (Entrance Lanes, Dumpster Pads)**

- 2 inches Asphaltic Concrete (FDOT Superpave Mix SP-12.5)
- 8 inches Crushed Limerock or Graded Aggregate Base (minimum LBR 100)
- 12 inches Stabilized Subgrade (minimum LBR 40)

The above recommended pavement sections represent minimum design thicknesses and, as such, periodic maintenance should be anticipated. Also, these recommended pavement sections should be confirmed or modified by your Civil Engineer, based on actual traffic and the owner's requirements. The pavement section materials and construction should comply with the Florida DOT and local municipality requirements.

**If the seasonal high groundwater level will be less than 24 inches below the bottom of the base course, we recommend the medium duty pavement section be used.**



**Warranty and Limitations of Study**

Our professional services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties, either expressed or implied. Magnum Engineering, Inc. is not responsible for the independent conclusions, opinions or recommendations made by others based on the field exploration and laboratory test data presented in this report.

Soil conditions at other locations may differ from those encountered in the test borings, and the passage of time may cause the soils conditions to change from those described in this report.

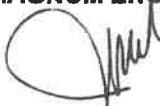
This report is intended for use by the designers of this project. While we have no objections to it being provided for review by parties to this project, it is not a specification document and is not to be used as a part of the specifications. If desired, we can assist in the development of specifications for this project based upon our exploration.

The nature and extent of variation and change in the subsurface conditions at the site may not become evident until the course of construction. Construction monitoring by the geotechnical engineer or his representative is therefore considered necessary to verify the subsurface conditions and to check that the soils connected construction phases are properly carried out. If significant variations or changes are in evidence, it may be necessary to reevaluate the recommendations in this report.

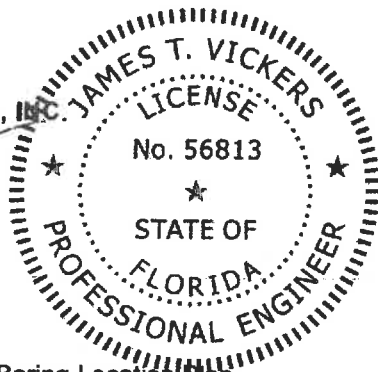
Furthermore, if the project characteristics are altered significantly from those discussed in this report, or if the project information contained in this report is incorrect and additional information becomes available, a review must be made by this office to determine if any modifications in the recommendations will be necessary.

We hope this letter provides sufficient information for the present. If you have any questions or comments, please feel free to call.

Sincerely,  
MAGNUM ENGINEERING, INC.



JAMES T. VICKERS, P.E.  
Sr. Geotechnical Engineer  
Florida Reg. #56813



Attachments: Figure #1 – Boring Location Plan  
Figure #2 – Logs of Borings

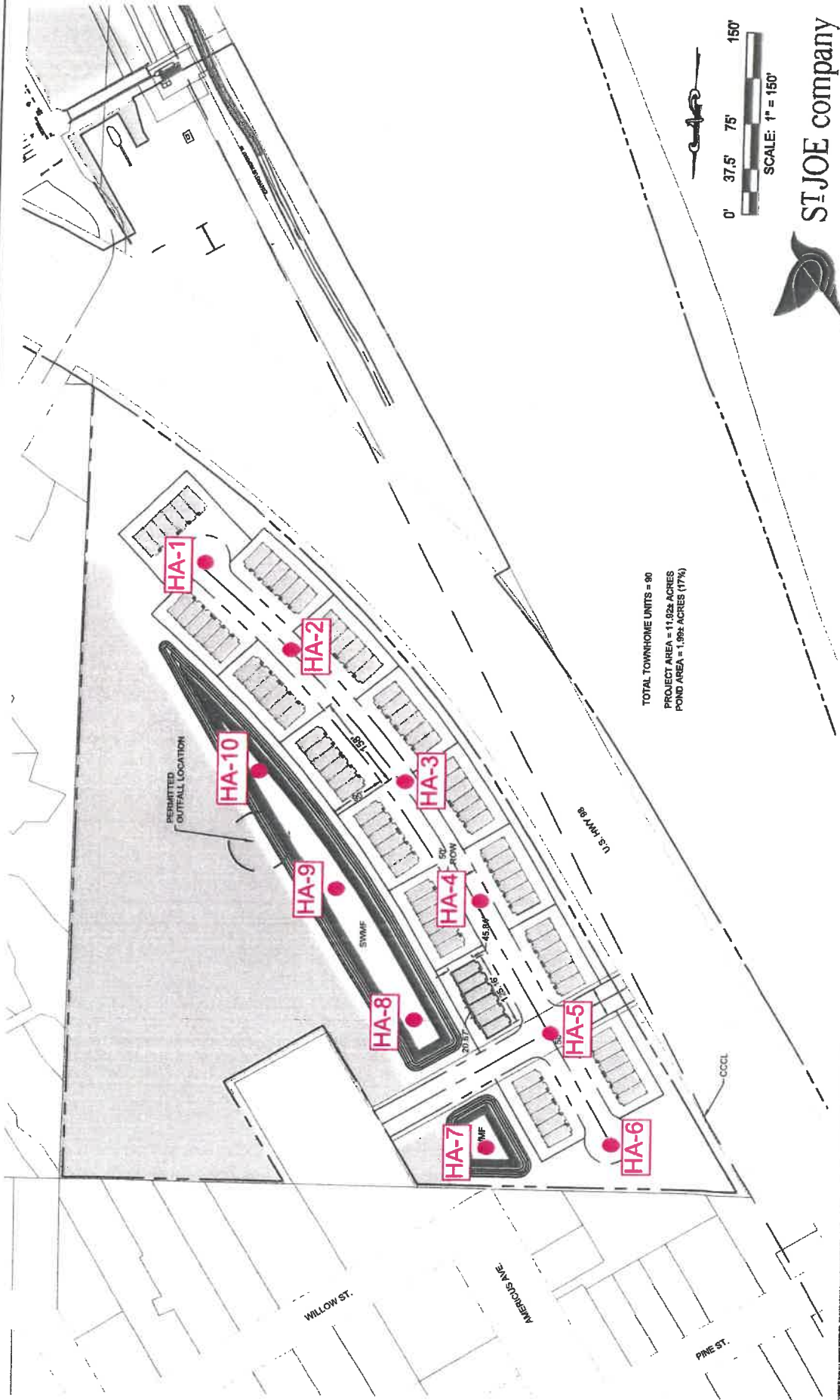


**MAGNUM ENGINEERING INC**  
**GEOTECHNICAL ENGINEERING**  
**CONSULTANTS**

## **BORING LOCATION PLAN**

**FIGURE # 1**

96



TOTAL TOWNHOME UNITS = 90  
PROJECT AREA = 11.92± ACRES  
POND AREA = 1.99± ACRES (17%)

REVISIONS		DATE	BY	APP'D
1				
2				
3				
4				
5				
6				

ESB 0007204	DATE: JUNE 2021	PROJECT NO.
	SCALE: 1" = 150'	000.000
	DRAWN: JWB	SHEET
	APPROVED: JWB	V1

<b>Dewberry</b> 203 Aberdeen Parkway, Panama City, FL 32405 850.522.0644, www.dewberry.com	WINDMARK TOWNHOMES CONCEPT THE ST. JOE COMPANY
--	---

January 10, 2023

Jim Anderson, City Manager  
City of Port St. Joe  
305 Cecil G. Costin, Sr. Blvd  
Port St. Joe, FL 32456


Dear Jim,

Norton Family Properties, LLC, et al, partnership owns the building and property located at 201 W. Hwy 98 in Port St. Joe. At present we are under contract to sale this property to Mr. and Mrs. Walter Woodrick. As you're already aware, a 15' wide alleyway exists along the northwestern boundary of our property, between mine and St. Joe Company owned properties. By all indications and logical observations, this narrow alley has not been utilized at all for many decades, and certainly no longer serves any functional purpose. Our plat shows that a corner of the existing building is set into the alleyway approximately 4" (inches). The prospective new owner would like some clarification that the city has no intentions of requiring the 4" portion of the building for being removed from the City's Right of Way (ROW). Can you please give us a letter from the City confirming the same?

I've enclosed for your convenience a copy of the survey that shows an intrusion that has existed since the 1940s.

Thank you in advance for your assistance!

Most sincerely,



Jim Norton,  
For the partners





## Current City Projects 1/17/23

- Maddox Park Drainage- Need more info. from engineer to bid and direction from the Board
- Clifford Sims Park Repairs- The Bids have been sent to FEMA/FDEM for approval
- Maddox Park Gazebo- Under Construction.
- Centennial Bldg. Rehab- Bids to be opened on 8/12/22, received one high bid. Working on new bid specs.
- Lighthouse Complex Rehab- Bids to be opened on 8/12/22. No Bids received. Working on new bid specs.
- Core Park Stage, Splash Pad, & Restroom- Was not Approved
- Sewer Rehab. CDBG-DR- Grant Funding Approved 5/21, Grant Agreement Received, working on Task Orders.
- City Hall Complex- Working on USDA Grant/Loan Docs. Currently on hold.
- Community Garden- Lease Agreement signed
- First Street Paving from First Street to Hwy 71 SCOP- Under Construction, Waiting on Stormwater Pipe.
- Long Ave Water/Sewer- Under Construction
- Long Ave. Paving- Start Date 1/3/23
- Ave. C, & D Paving- Start Date 1/3/23
- Dr. Joe Parking Lot Paving- Change Order Approved to include in Long Ave.
- New Boat Ramp Access Road- Tabled
- Beacon Hill Sewer- The Lift Station Contract has been signed and Design is ongoing for the Sewer Main.
- Skate Park- Working with the School on the location
- Washington Gym Roof Flashing & Workforce Bldg. Roof Coating- Working on Quotes
- Cleaning of Ponds on Baltzell and at Core Park – Underway
- ESAD Sewer Purchase Evaluation for Upgrades- Underway
- Buck Griffin Lake- Repair of the Spill Over is Underway
- 20<sup>th</sup> Street Stormwater Pipe- Working on Camera Quote

**PORT ST JOE POLICE DEPARTMENT**

***Certifications for incentive pay***

**FIELD TRAINING OFFICER- 2 Officers**

40 Hour Certification

**INTOXILYZER AGENCY INSPECTOR- 2 Officers**

40 Hour Certification

**CRIMESCENE/EVIDENCE MANAGEMENT- 1 Officer**

40 hour training

**BASIC LAW ENFORCEMENT INSTRUCTOR- 1 Officer**

64 Hours+

**Licenses Required by Department  
Public Works**

**Water Department**

1. FDEP Level III Water Distribution License, to be held by 2 employees
2. Class B CDL, to be held by 2 employees

**Sewer Department**

1. Class B CDL, to be held by 2 employees
2. FDEP Sewer Collection Level III License (when required by the State), to be held by 2 employees

**Parks and Recreation**

1. State of Florida Ornamental Turf Pesticide/Herbicide License, to be held by 2 employees
2. State of Florida Aquatic Herbicide License, to be held by 2 employees (potentially future)

**Streets and Highways**

1. Florida DOC inmate supervisor certification, to be held by 2-3 employees
2. Class B CDL, to be held by the Equipment Operator

City of Port St. Joe, Florida

Petitioner,

Case No. 20191000117

vs.

Timothy Elder  
12586 Hwy 12 South  
Bristol, FL 32321

Respondent

**ORDER OF THE SPECIAL MAGISTRATE**

THIS MATTER was heard on August 23, 2022 before the Special Magistrate on the Petition of the City of Port St. Joe. Present for the hearing was Richard Burkett, Code Inspector. Respondent did not appear. Upon taking testimony of the Code Inspector and reviewing the evidence, the Special Magistrate finds as follows:

1. Respondent was properly served with notice of the alleged violation and notice of the hearing by certified mail and by publication.
2. Code Inspector Burkett described the violation and presented photographic evidence of the alleged violation, as well as testimony as to the condition of the property.
3. Respondent's property located at 1310 Long Ave., Port St. Joe, FL 32456 is found to be uninhabitable and is structurally unsafe and a threat to public safety.
4. Respondent's subject property is found to be in violation of Ordinance 296 of the City Code (the Code).

**IT IS THEREFORE ORDERED AS FOLLOWS:**

- A. Respondent shall have forty-five (45) days from the date of this Order to begin demolition of the property and shall have ninety (90) days from the date of this Order to complete demolition and all required cleanup of the property.

B. Respondent shall contact the Code Inspector (850-229-8261; ext.112) upon completion of the above-ordered items for re-inspection of the premises and further actions consistent with this Order.

C. Respondent is assessed costs as follows:

(1) Notice of violation postage: \$ 6.90

(2) Notice of hearing postage: \$ 6.90

(3) Cost of publication: \$ 50.00

(4) Code Inspector time: \$ 100.00

**Total: \$ 163.80**

D. If Respondent fails to correct the Code violation cited herein within the specified time, the City of Port St. Joe shall have the right to take all reasonable and necessary actions to bring Respondent's property into compliance with this order, including having the property demolished and made safe and to assess the costs of said action against Respondent.

E. All costs assessed herein, including the costs the City may incur to bring the property in compliance with this Order shall constitute a lien on the identified property located at 1310 Long Ave., Port St. Joe, FL, Parcel no. 05171-000R and this Order shall be recorded in the Public-Records of Gulf County, Florida.

F. Any party to this action may appeal this Order to the Circuit Court of the 14<sup>th</sup> Judicial Circuit for Gulf County, Florida. Any such appeal shall be filed within thirty (30) days of the date of this Order.

G. The Magistrate may enter such further orders to allow the City to recover any costs of demolition the subject structure.



DATED this 31<sup>st</sup> day of August 2022.

By: Mel Magidson Jr.

Hon. Mel Magidson Jr.  
City of Port St. Joe Special Magistrate

Copy:

Timothy Elder  
12586 Hwy 12 South  
Bristol, FL 32321

Richard Burkett  
City of Port St. Joe  
Code Inspector

City vs. Timothy Elder

Address 1310 Long Ave Street Gulf County Parcel 05171-000R

The hearing was held on August 23, 2022, after being properly noticed. The owners did not attend the hearing.

The property owner was given 45 days from August 31, 2022, to bring the property into compliance or begin demolition. As of December 21, 2022, no work has been done and continues to be in violation.

Requested actions

1. Request for bids to demolish the structure and place a lien on the property for all costs involved in the cleanup.

City of Port St. Joe, Florida  
Petitioner,

Case No. 2021050011

vs.

James Padgett  
379 E Bayview Dr.  
Eastpoint, FL 32328

Respondent

**ORDER OF THE SPECIAL MAGISTRATE**

THIS MATTER was heard on March 3, 2022, before the Special Magistrate on the Petition of the City of Port St. Joe. Present for the hearing was Richard Burkett, Code Inspector. Respondent did not appear. Upon taking testimony of the Code Inspector and reviewing the evidence, the Special Magistrate finds as follows:

1. Respondent was properly served with notice of the alleged violation and notice of the hearing by certified mail and by publication.
2. Code Inspector Burkett described the violation and presented photographic evidence of the alleged violation, as well as testimony as to the condition of the property and the need for a new roof.
3. Respondent's property located at 903 16<sup>th</sup> Street, Port St. Joe, FL, parcel no: 05441-000R is found to be uninhabitable and is structurally unsafe and a threat to public safety.
4. The subject property was condemned by FEMA as a substandard property after Hurricane Michael.
5. Respondent's subject property is found to be in violation of Ordinance 296 of the City Code (the Code).

6.

IT IS THEREFORE ORDERED AS FOLLOWS:

A. Respondent shall have sixty (60) days from the date of this Order to begin to bring the subject property into compliance with the Code but replacing the damaged roof or begin demolition of the subject property and shall have ninety (90) days from the date of this Order to complete demolition and all required cleanup of the property.

B. Respondent shall contact the Code Inspector (850-229-8261; ext. 112) upon completion of the above-ordered items for re-inspection of the premises and further actions consistent with this Order.

C. Respondent is assessed costs as follows:

(1) Notice of violation postage: \$ 6.90

(2) Notice of hearing postage: \$ 6.90

(3) Cost of publication: \$ 50.00

(4) Code Inspector time: \$ 100.00

(5) Clerk of Court filing fee \$ 27.00

**Total:** \$ 190.80

D. If Respondent fails to correct the Code violation cited herein within the specified time, the City of Port St. Joe shall have the right to take all reasonable and necessary actions to bring Respondent's property into compliance with this order, including having the property demolished and made safe and to assess the costs of said action against Respondent.

E. All costs assessed herein, including the costs the City may incur to bring the property



in compliance with this Order shall constitute a lien on the identified property located at 903 16<sup>th</sup> Street, Port St. Joe, FL, Parcel no. 05441-000R and this Order shall be recorded in the Public Records of Gulf County, Florida.

F. Any party to this action may appeal this Order to the Circuit Court of the 14<sup>th</sup> Judicial Circuit for Gulf County, Florida. Any such appeal shall be filed within thirty (30) days of the date of this Order.

G. The Magistrate may enter such further orders to allow the City to recover any costs of demolition the subject structure. DATED this 8th day of March, 2022.

By: Mel Magidson Jr.

Hon. Mel Magidson Jr.  
City of Port St. Joe Special Magistrate

Copy:

James Padgett  
379 E Bayview Dr.  
Eastpoint, FL 32328

Richard Burkett  
City of Port St. Joe  
Code Inspector

**City vs. Natalie Franks & James Padgett**

**Address 903 16<sup>th</sup> Street Gulf County Parcel 05441-000R**

**The hearing was held on March 3, 2022, after being properly noticed. The owners did not attend the hearing.**

**The property owner was given 90 days from March 8, 2022, to bring the property into compliance or begin demolition. As of December 21, 2022, no work has been done and continues to be in violation.**

**Requested actions**

- 1. Request for bids to demolish the structure and place a lien on the property for all costs involved in the cleanup.**

CITY OF PORT ST. JOE,

Petitioner,

Case No. 2022010001 A & B

vs.

JAMES MASSEY  
516 9<sup>th</sup> Street  
Port St. Joe, FL 32456

Respondent

**ORDER OF THE SPECIAL MAGISTRATE**

THIS MATTER was heard on July 12, 2022, before the Special Magistrate on the Petition of the City of Port St. Joe. Present for the hearing was Richard Burkett, Code Inspector. Respondent did not appear. Upon taking testimony and reviewing the evidence, the Special Magistrate finds as follows:

1. Code Inspector Burkett testified as to the violation and presented photographic evidence of the alleged violation.
2. Based upon the evidence, Respondent failed to comply with the Order in this matter dated July 15, 2021.

IT IS THEREFORE ORDERED AS FOLLOWS:

- A. Respondent shall have thirty (30) days from the date of this Order to bring the subject property into compliance with the Code.
- B. Code Enforcement Officer shall inspect the property at the end of the 30-day period to determine the status of the property and shall report to this court, along with photographs of the subject property, to determine if Respondent has complied with the Code.
- C. The fines that were previously imposed by this court shall be abated if Respondent

complies with this Order; however, if Respondent fails to comply with this Order, said fines will be reinstated as ordered.

D. Respondent is assessed costs as follows:

- (1) Notice of violation postage: \$ 6.90
- (2) Notice of hearing postage: \$ 6.90
- (3) Cost of publication: \$ 50.00
- (4) Code Inspector time: \$ 100.00
- (5) Clerk of Court filing fee \$ 27.00
- Total: \$ 190.80**

- E. If Respondent fails to comply with this Order within the specified time, the City of Port St. Joe shall have the right to take all reasonable and necessary actions to bring the subject property into compliance with the Code and to assess the costs of said action against Respondent.
- F. All costs assessed herein, including the costs the City may incur to bring the property in compliance with this Order shall constitute a lien on the identified property located at 516 9<sup>th</sup> Street, Port St. Joe, FL, Parcel no: 05103-000R and this Order shall be recorded in the Public Records of Gulf County, Florida.
- G. Any party to this action may appeal this Order to the Circuit Court of the 14<sup>th</sup> Judicial Circuit for Gulf County, Florida. Any such appeal shall be filed within thirty (30) days of the date of this Order.
- H. The Magistrate may enter such further orders to allow the City to recover any costs of demolition the subject structure.

DONE AND ORDERED this 20<sup>th</sup> day July, 2022.

Mel Magidson

Mel Magidson Jr.  
City of Port St. Joe Special Magistrate

Copy:

JAMES MASSEY  
516 9<sup>th</sup> Street  
Port St. Joe, FL 32456

Richard Burkett  
City of Port St. Joe  
Code Inspector



**City vs. James Massey**

**Address 516 9th Street Gulf County Parcel 05103-000R**

The hearing was held on July 12, 2022, after being properly noticed. The owner did not attend the hearing.

The property owner was given 30 days from July 20, 2022, to bring the property into compliance or begin demolition. As of December 21, 2022, no work has been done and continues to be in violation.

**Requested actions**

1. Request for bids to move both sheds to within owners' property lines and reinstall anchors per the building codes and place a lien on the property for all costs involved in the cleanup.

B2021030001

AE9'

**EPCI**  
**PORT ST JOE BUILDING DEPARTMENT**

**APPLICATION FOR BUILDING PERMIT**

DATE: 2-3-2020 Permit # \_\_\_\_\_ Permit Fee \_\_\_\_\_

OWNER'S NAME: James Massey

ADDRESS: 516 9<sup>th</sup> St., P.S.J., FL 32456

CITY, STATE & ZIP CODE: P.S.J. 32456 PHONE # 899-2042

FEE SIMPLE TITLE HOLDER (IF OTHER THAN OWNER): \_\_\_\_\_

ADDRESS: \_\_\_\_\_

CITY, STATE & ZIP CODE: \_\_\_\_\_ PHONE # \_\_\_\_\_

CONTRACTOR'S NAME: Bestway Portable Building S

ADDRESS: 2919 N. Hwy 231

CITY, STATE & ZIP CODE: P.C., FL 32405 PHONE # 747-1984

STATE LICENSE NUMBER: RR288811422 COMPETENCY CARD # \_\_\_\_\_

ADDRESS OF PROJECT: 516 9<sup>th</sup> St. P.S.J.

PROPOSED USE OF SITE: 12' x 20' Shed

WILL THE STRUCTURE BE LOCATED AT LEAST 30 FEET FROM ANY BODY OF WATER?  
☒ YES ☐ NO

PROPERTY PARCEL ID # 05103-000R

LEGAL DESCRIPTION OF PROPERTY: City of PSJ Lot 16 & E/2 of Lot 14 FR Land BIK 52 MAP 50A ORB 239/42

**IF THE APPLICATION IS FOR A COMMERCIAL PROJECT PLEASE LIST THE NAME OF THE BUSINESS:**

BONDING COMPANY: \_\_\_\_\_

ADDRESS: \_\_\_\_\_ CITY, STATE & ZIP: \_\_\_\_\_

ARCHITECT'S/ENGINEER'S NAME: \_\_\_\_\_

ADDRESS: \_\_\_\_\_ CITY, STATE & ZIP: \_\_\_\_\_

MORTGAGE LENDER'S NAME: \_\_\_\_\_

ADDRESS: \_\_\_\_\_ CITY, STATE & ZIP: \_\_\_\_\_

WATER SYSTEM PROVIDER: \_\_\_\_\_ SEWER SYSTEM PROVIDER: \_\_\_\_\_

PRIVATE WATER WELL: \_\_\_\_\_ SEPTIC TANK PERMIT NUMBER: \_\_\_\_\_

Port St. Joe Plan Review

Review Date: 2-3-20

Reviewed By: RW

Owner: Hassey

Contractor: Tool Time

Address: 516 9<sup>th</sup> St.

Parcel ID: 05103-000R

Residential: ☒

Commercial: ☐

Project

Description: 12 X 20 (240 sf) storage shed

Type of Development Order: ---

Zoning: R-2B Density Units allowed: --- Proposed Density Units: ---

Flood Zone: AE 9 Seaward of CCCL? --- Date of DEP permit if required: ---

Elevation First Finished Floor: ---

292 ac =

Lot Size: 75 X 170 12,720<sup>0</sup> sf Covered Area Sq. Ft.: 3,244

House H/C: 1720 House Footprint: --- Garage: ---

Porches: 168 sf Deck/patio: 396 Shed: ---

Pool/Decking: --- Driveway: 480 sf Other: ---

Height Allowed: 60' Height Proposed: 8'

Impervious Surface Allowed: 60% Proposed: 25.5%

Setbacks required: Front: 20 Rear: 20 Left: 10' Right: 10'

Setbacks proposed: Front: 92' Rear: 55' Left: 23' Right: 40'

Notes: Gulf PA = 2,284 House coverage +(old shed) 240 +(new shed) 240+  
480 driveway = 3,244 sf total

\*  
Recommendations: Need flood vent openings (or elevated structure)  
since AE 9 (was in X) (paperwork said X flood zone)

Permit Number \_\_\_\_\_  
Parcel ID Number 05103-000R

### NOTICE OF COMMENCEMENT

State of Florida  
County of Gulf

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of Property (legal description): City of P.S.T. Lot 16 & E/4 of Lot 14 ORB 239/442 FR LAMB  
A) Street (job) Address: 516 9th St. P.S.T., FL 32456 81K 53 MAP SCA
2. General description of improvements: 13' X 20' shed
3. Owner Information or Lessee information if the Lessee contracted for the improvement:  
A) Name and address: James Massey 516 9th St.  
B) Name and address of fee simple titleholder (if different than Owner listed above) \_\_\_\_\_  
C) Interest in property: 100%
4. Contractor Information  
A) Name and address: Bestway Portable Buildings Inc.  
B) Telephone Number: 2519-BWY-231N Fax No.: (optional) \_\_\_\_\_  
C) Address: Panama City, FL 32405
5. Surety (if applicable, a copy of the payment bond is attached)  
A) Name and address: \_\_\_\_\_  
B) Telephone No.: \_\_\_\_\_  
C) Amount of Bond: \$ \_\_\_\_\_
6. Lender  
A) Name and address: \_\_\_\_\_  
B) Telephone No.: \_\_\_\_\_
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:  
A) Name and address: \_\_\_\_\_ Fax No.: (optional) \_\_\_\_\_  
B) Telephone No.: \_\_\_\_\_
8. (a) In addition to himself or herself, Owner designates \_\_\_\_\_ of \_\_\_\_\_  
to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes  
(b) Phone Number of Person or entity designated by Owner: \_\_\_\_\_
9. Expiration date of notice of commencement (the expiration date will be one (1) year from the date of recording unless a different date is specified): .20

**WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART 1, SECTION 713.13, FLORIDA STATUTES AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.**

James Massey  
(Signature of Owner or Lessee, or Owner's or Lessee's)



James Massey / owner  
(Print Name and Provide Signatory's Title/Office)

The foregoing instrument was acknowledged before me this 31 day of January, 2020  
by Jason Moulder as Notary (type of authority, e.g. officer, trustee, attorney in fact) for James Massey as owner (Name of Person) (type of authority, e.g. officer, trustee, attorney in fact)  
for \_\_\_\_\_ (name of party on behalf of whom instrument was executed).

Personally Known ☐ Produced ID ☒

Signature of Notary Public - State of Florida

Type of ID D.C.

Print Name



# EPCI

## Product Affidavit

I Michael Koppel hereby certify that all products used in building at the  
property address, 516 9th St. meet the requirements of the Florida Building  
Code for Product Approval.

Tina Huguen  
Signature

State of Florida

County of Gulf

Sworn to and subscribed before me this 3rd day of Feb 20 20

By Tina Huguen

Erika McNair

Notary Public, State of Florida

Personally known        or

Produced Identification       

Type of identification produced PLDL







Gulf County,

qPublic.net

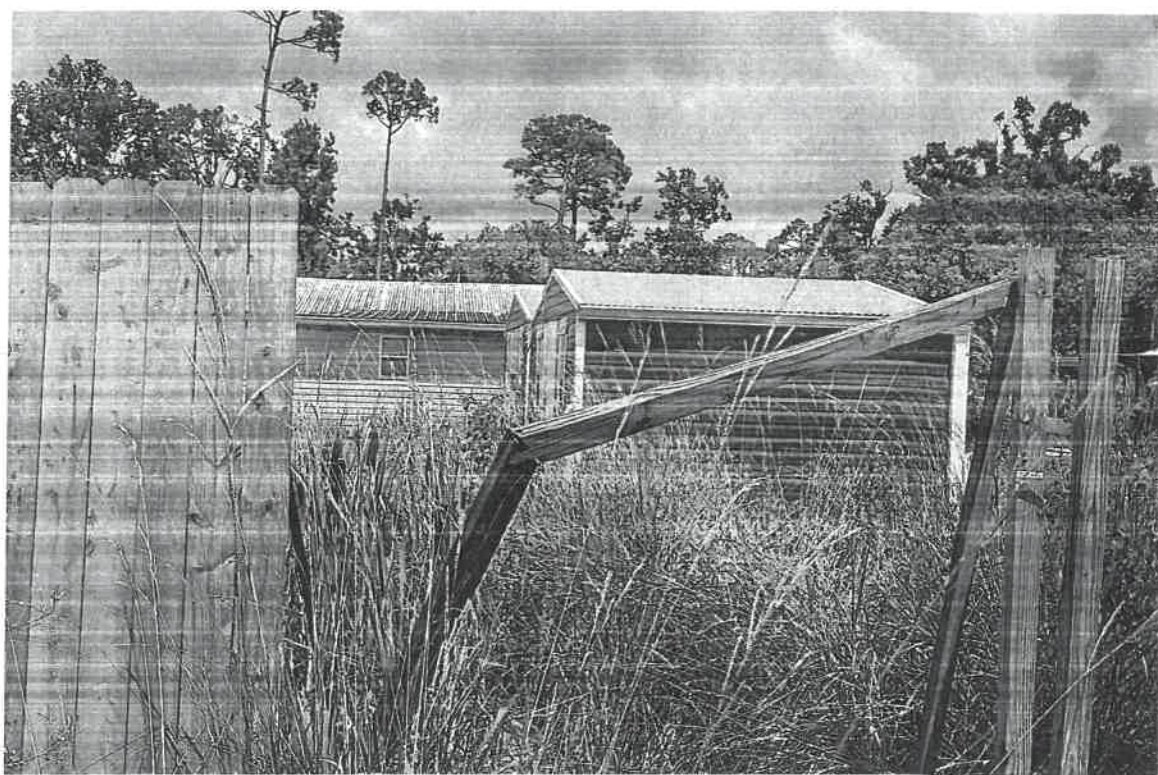
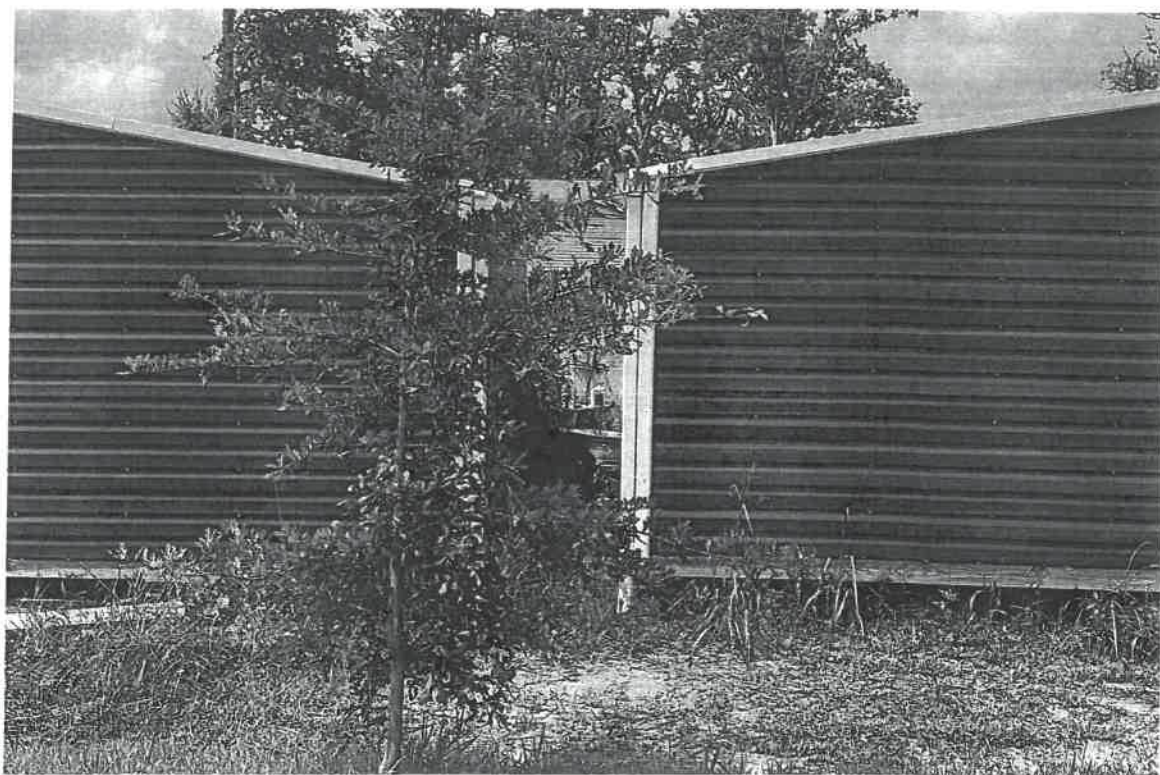


James Massey

Parcel # 05103-000R  
516 9th St. 118

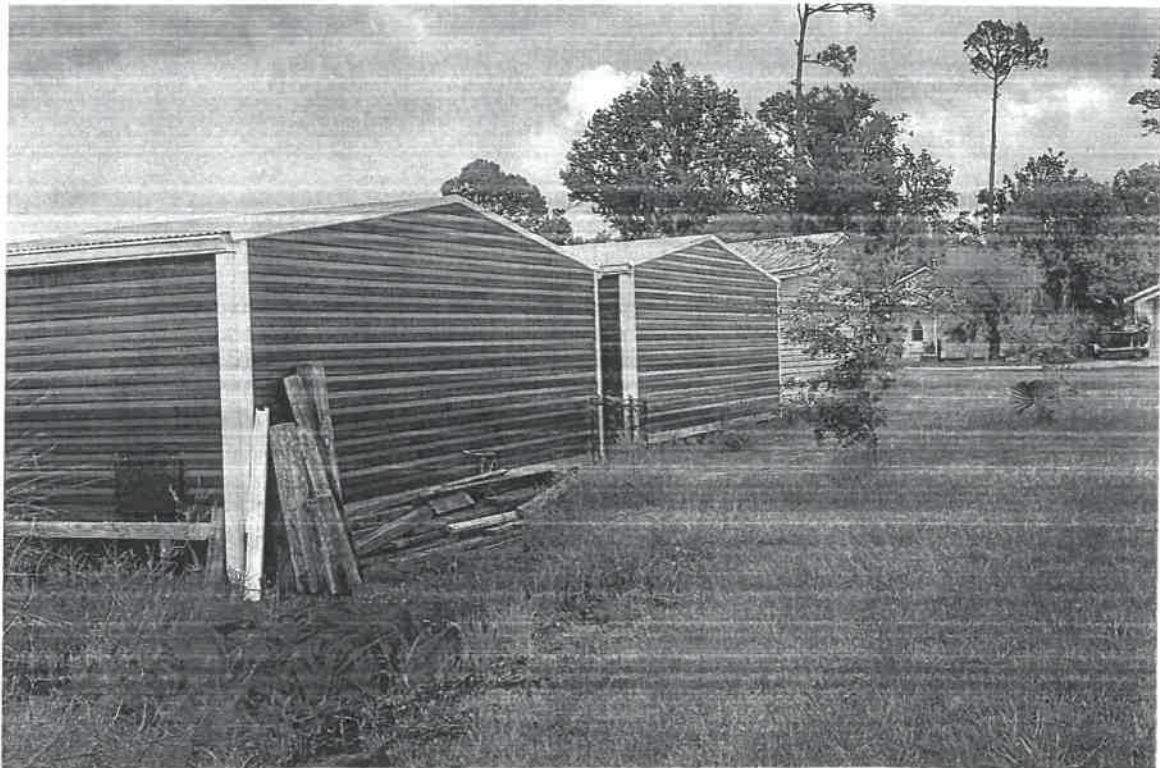
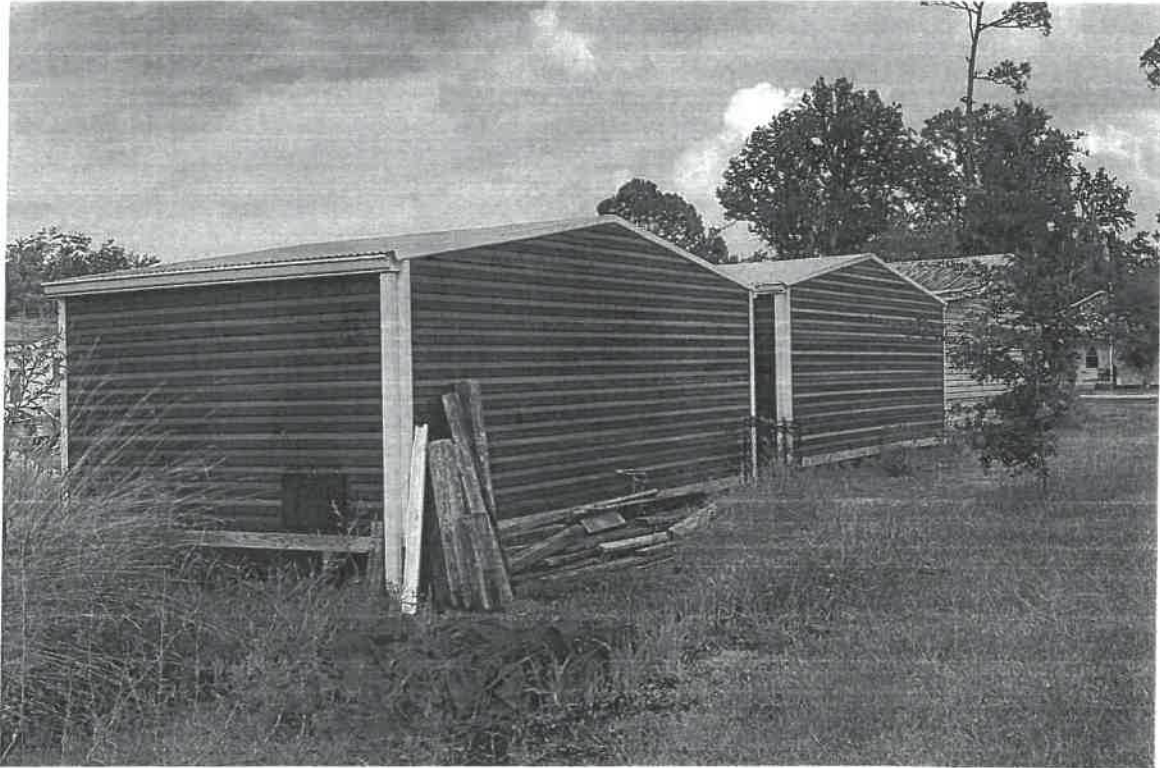


B2021030001 3-8-21 516 9th St G. duess

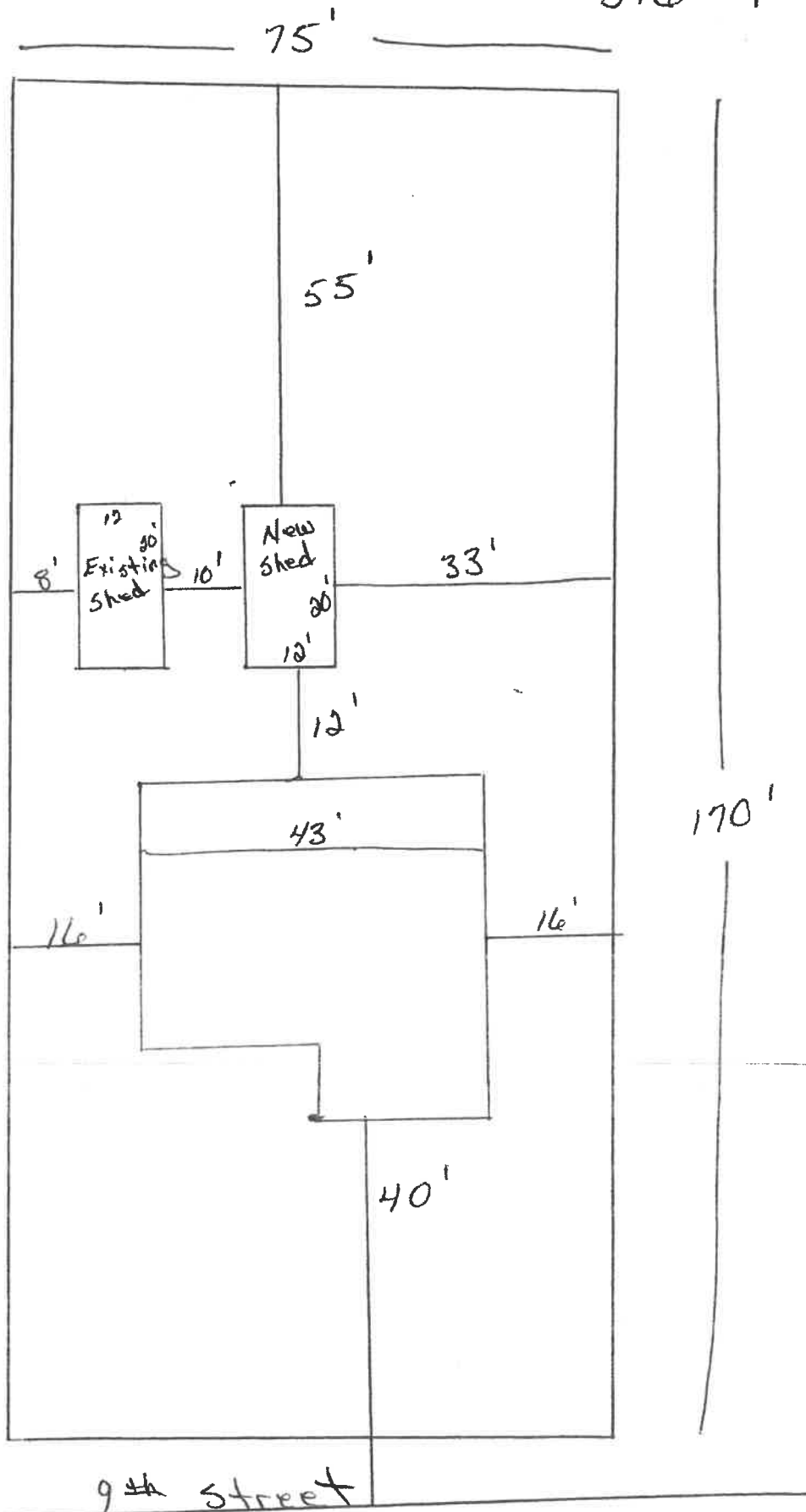




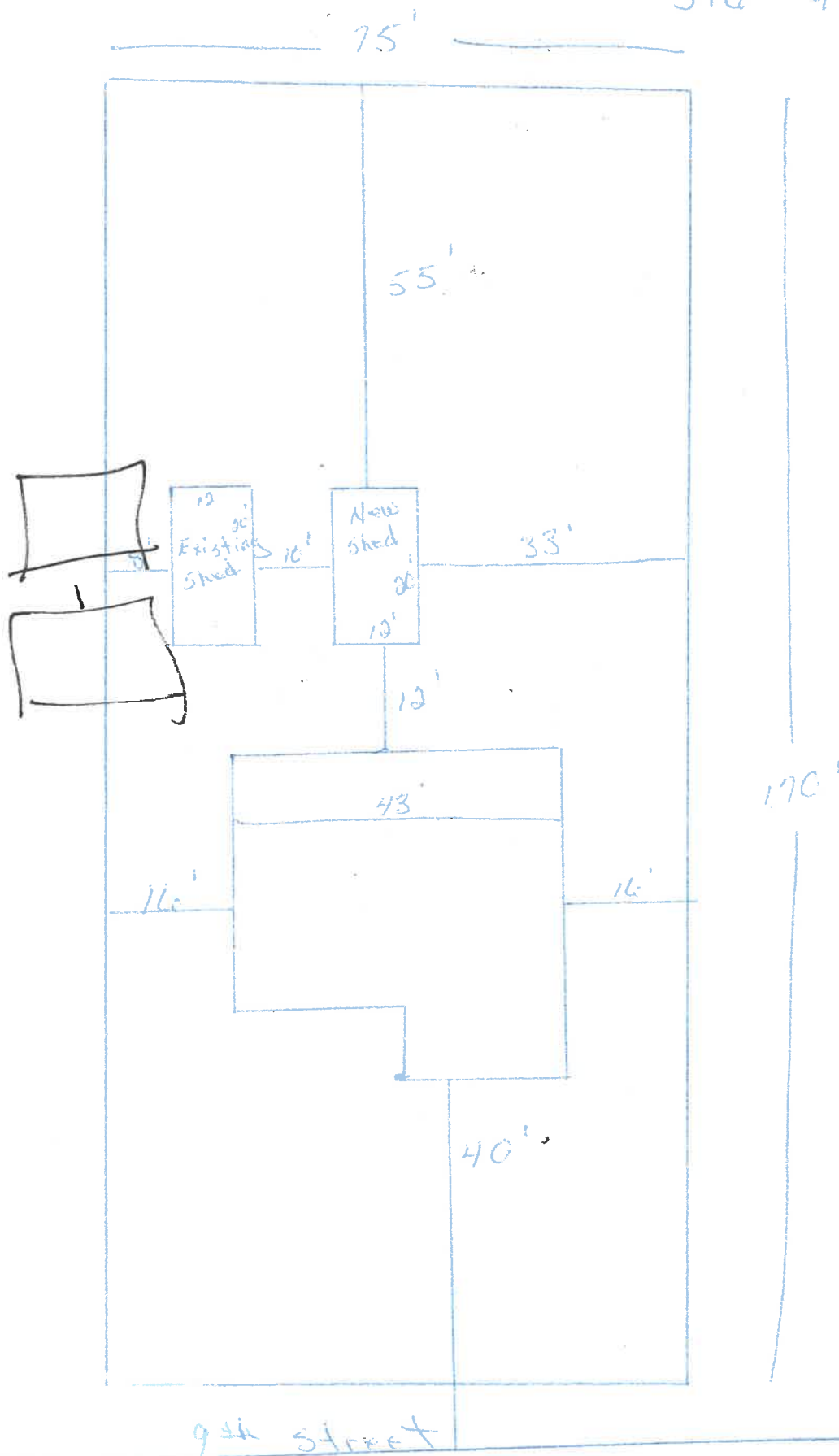
B2021030001 3-8-21 516 9th St Bridgeport



Parcel # 05103-000K  
James Massey  
516 9th Street



Parcel # 05103-0008  
James Massey  
516 9th Street





The City of Port St. Joe realizes that inability to work because of illness or injury may cause a serious economic hardship. The City provides a plan of sick leave to ensure a continuation of pay for employees who are temporarily unable to perform their duties because of illness or injury.

Each regular full-time employee will earn 8 hours sick leave with pay, each month; the unused portion of which shall be cumulative.

Leave time accrued to the benefit of an employee shall be subject to the following regulations:

- A. No cap on the amount of accumulation of sick leave.
- B. If an employee has no sick leave available then he/she will be required to use annual leave prior to leave without pay. If the employee doesn't have annual leave then the employee will be required to take leave without pay. An employee will not be allowed to take leave without pay if he/she has sick or annual leave available.
- C. Days which an employee is absent from work as a result of any illness, disease or injury covered by the State Worker's Compensation law or as a result of any provision of such law as applied to such illness, disease or injury shall be charged against sick leave until such time as the worker's compensation begins payment of a salary. If employee has exasperated all earned sick leave then he/she will be permitted to use earned annual leave.
- D. Sick leave may be authorized during the employee's probationary period. However, in the event the employee resigns or is otherwise terminated before the end of the first six (6) month period of employment, any sick leave taken will be reimbursed to the City by deduction from the employee's final pay.
- E. Sick leave will not be granted in advance of accrual.
- F. Sick leave will not be considered as time worked for overtime computation.
- G. Sick leave can be charged in increments of no less than one quarter of an hour.
- H. Should a holiday occur during sick leave, the holiday shall be charged to holiday leave.

To receive compensation while absent on sick leave, the employee shall notify his/her immediate supervisor or department head in accordance with department regulations.

The department head or immediate supervisor may request a physician's certificate to verify the illness of any employee on sick leave if there is reasonable cause to believe that the employee is abusing the intent of sick leave.

Sick leave may be granted for the following purposes:

- A. Personal injury, pregnancy or illness of the employee.
- B. Medical, dental, optical or chiropractic examination or treatment when it is not possible to arrange the appointment for off-duty hours

## Grants Updated- 1/17/23

Title	Amount	Status
NWFWMD/NERDA	\$971,850	Draft Stormwater Master Plan Complete. Water Quality portion is on hold.
FDOT/SCOP	\$397,375	Application for resurfacing of first Street from Hwy 98 to Hwy 71. Approved for 21/22 funding
FEMA PA	\$9,778,787	Damage from Hurricane Michael
FDEM	\$660,943 \$5,000	Hazard Mitigation. Elevation of (12) lift stations and switch gear for Washington Gym Generator Power. Submitted 3/6/20. 25% match. Approved 12/16/22
FRDAP	\$100,000	Applied for (2) grants on 10/15/20. Washington Gym Complex and Dodder Parker Park for exercise equipment, playground Equipt. & Disc Golf. Grants approved 7/21 Waiting on State Approval to Award Bids
FDEP/SRF	\$4,537,600	Application submitted for Construction of Long Ave. Sewer Line and Lift Station. Grant \$3,630,080 and Loan \$907,520 Combo 80/20. Approved, Notice of Award has been issued.
Historic Resources/Hurricane Michael	\$497,495	Centennial Bldg. Rehab. Grant awarded out for Bids to be opened on 8/12/22
CDBG-DR	\$9,996,000	Sewer Rehab- City Wide. Approved 5/21.
CDBG-DR	\$8,566,469	Police/Fire Station. Application not approved. Re-submitted in Round two of funding on 9/15/21. Application not approved.
CDBG-DR	\$4,987,330	Intelligent Stormwater on MLK-FAMU. Application not approved.
FDOT/SCOP	TBD	Niles Rd. from Garrison to Long Ave Re-surfacing. Approved, waiting on Grant Agreement.
National Park System/Hurricane Michael	\$83,000	Washington Gym Rehabilitation. Submitted by UF. Approved and will be administered thru the State of Florida Division of Historical Resources
Historic Resources/Hurricane Michael	\$327,707	Cape San Blas Lighthouse Complex. Bids to be opened on 8/12/22.
USDA	\$4,000,000	Potential 65% loan/35% grant for new Government Complex
COVID-19 Rescue Plan	\$1,786,545	Grant Agreement signed 9/15/21. 50% disbursement received.
FDEO	\$675,426.00	Commercial District Waterline Replacement. Grant Application submitted 1/2022. Grant Approved 4/8/22.
FDEP Water Protection Funds	\$965,000	System Wide Septic to Sewer for 175 connections. Grant Application approved 11/10/21. Accepting Applications for service.
FDEP Water Protection Funds	\$4,300,000	Beacon Hill Sewer. Grant Application Approved 11/9/21. Dewberry is working on the Design.

FDEP Water Protection Funds	\$1,834,401.60	Pipe Replacement under the 10 <sup>th</sup> Street Park. Grant Application submitted 7/15/21. Was not approved.
CDBG- DR Phase II	\$6,654,566	Road & Stormwater Repairs. Application submitted 9/15/21. Application not approved.
FDEP	\$218,895	Resilient Florida (Study of PSJ). Submitted 8/30/21, Working with UF Approved, waiting on Grant Agreement
FRDAP	\$150,000	Core Park Splash Pad, 25% City Match. Submitted 10/14/21. Was not approved.
FDEP	\$145,000	Wastewater Treatment Plant Feasibility Study, submitted 8/22.
FDOT/SCOP	\$575,417.65	Application for re-surfacing Allen Memorial. Approved on 8/23/22 for the 2024 fiscal year.
Legislative Request Rep. Shoaf	\$500,000 \$2,000,000	Core Park Stage Field of Dreams, both submitted on 7/8/22
NOAA	\$280,000	Stormwater Management (H&H) Study submitted on 10/13/22
FDOT	\$100,000	Hwy 98 Beautification Grant, Approved 12/16/22